



CONTACT INFORMATION  
Mining Records Curator  
Arizona Geological Survey  
3550 N. Central Ave, 2nd floor  
Phoenix, AZ, 85012  
602-771-1601  
<http://www.azgs.az.gov>  
[inquiries@azgs.az.gov](mailto:inquiries@azgs.az.gov)

The following file is part of the Walter E. Heinrichs, Jr. Mining Collection

#### **ACCESS STATEMENT**

These digitized collections are accessible for purposes of education and research. We have indicated what we know about copyright and rights of privacy, publicity, or trademark. Due to the nature of archival collections, we are not always able to identify this information. We are eager to hear from any rights owners, so that we may obtain accurate information. Upon request, we will remove material from public view while we address a rights issue.

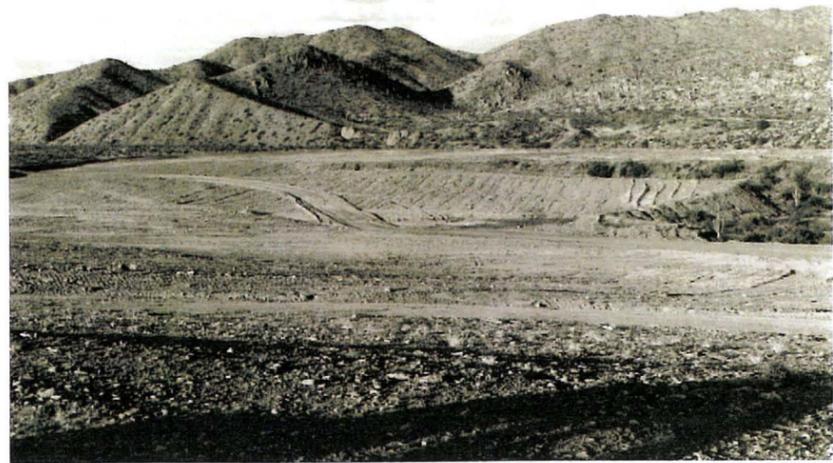
#### **CONSTRAINTS STATEMENT**

The Arizona Geological Survey does not claim to control all rights for all materials in its collection. These rights include, but are not limited to: copyright, privacy rights, and cultural protection rights. The User hereby assumes all responsibility for obtaining any rights to use the material in excess of "fair use."

The Survey makes no intellectual property claims to the products created by individual authors in the manuscript collections, except when the author deeded those rights to the Survey or when those authors were employed by the State of Arizona and created intellectual products as a function of their official duties. The Survey does maintain property rights to the physical and digital representations of the works.

#### **QUALITY STATEMENT**

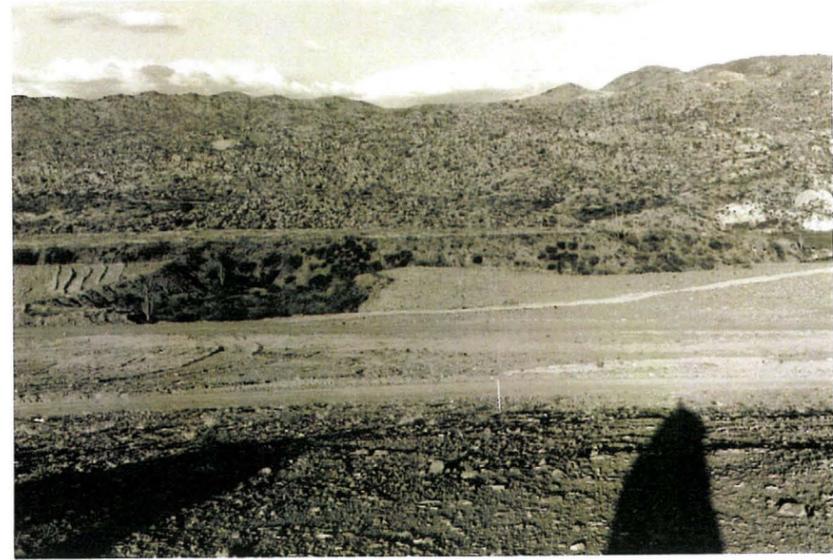
The Arizona Geological Survey is not responsible for the accuracy of the records, information, or opinions that may be contained in the files. The Survey collects, catalogs, and archives data on mineral properties regardless of its views of the veracity or accuracy of those data.



APR • 65



APR • 65



APR • 65



APR • 65



APR • 65



APR • 65



APR • 65



APR • 65



APR • 65



APR • 65



APR • 65



APR • 65



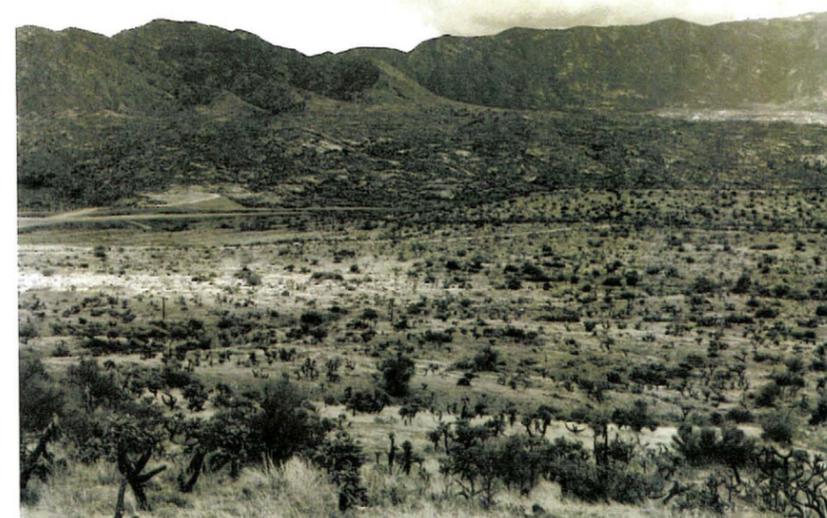
APR • 65



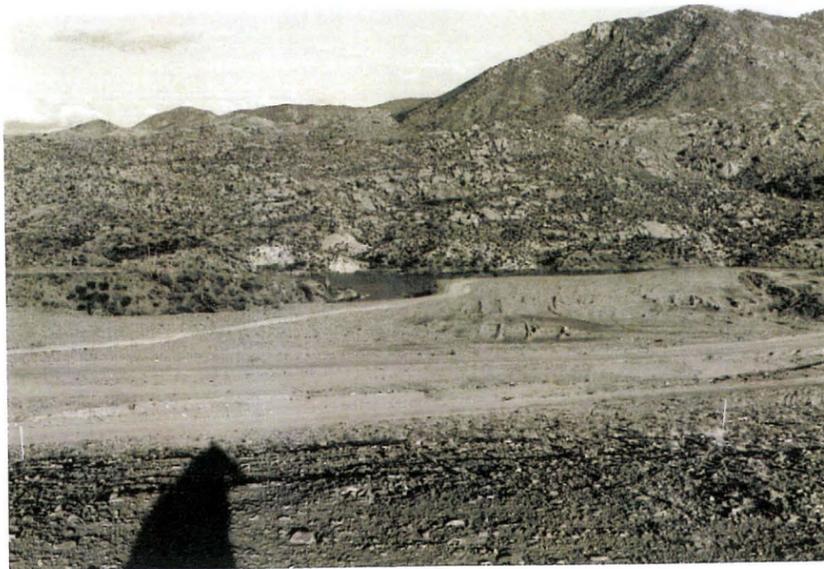
APR • 65



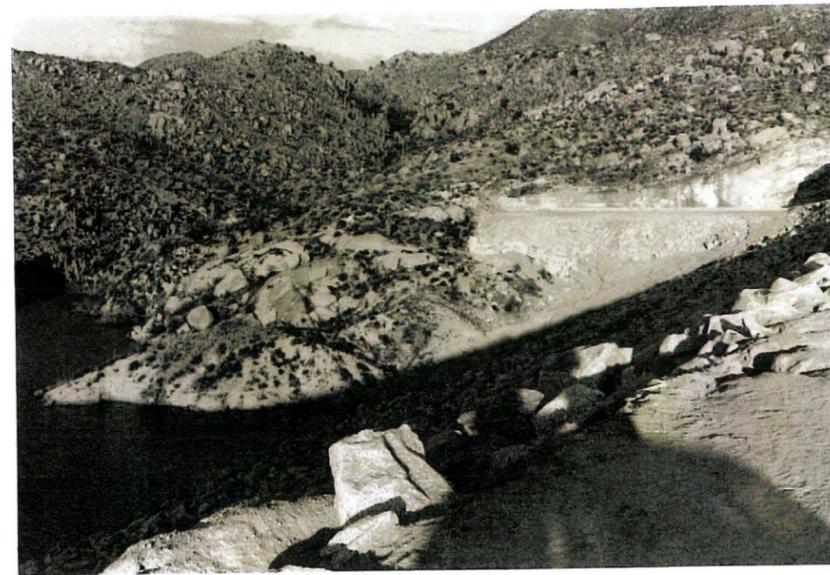
APR • 65



APR • 65



APR • 65



APR • 65



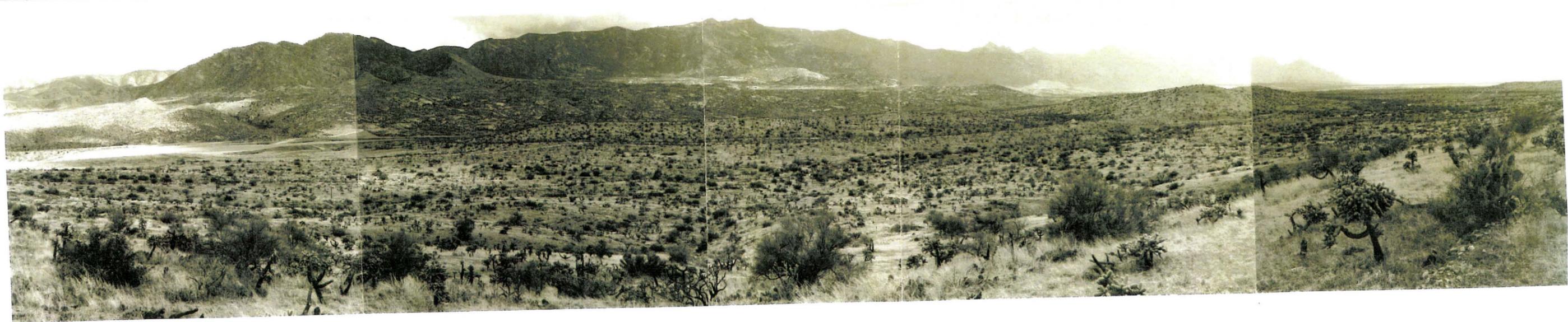
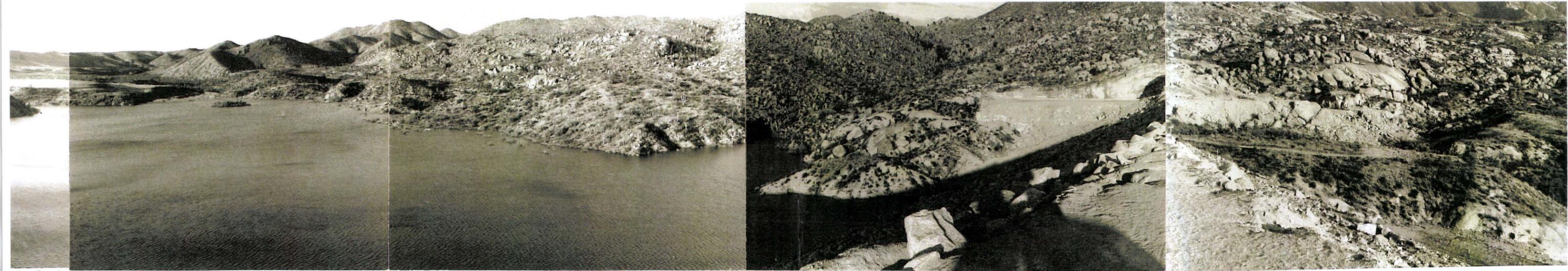
APR • 65



APR • 65

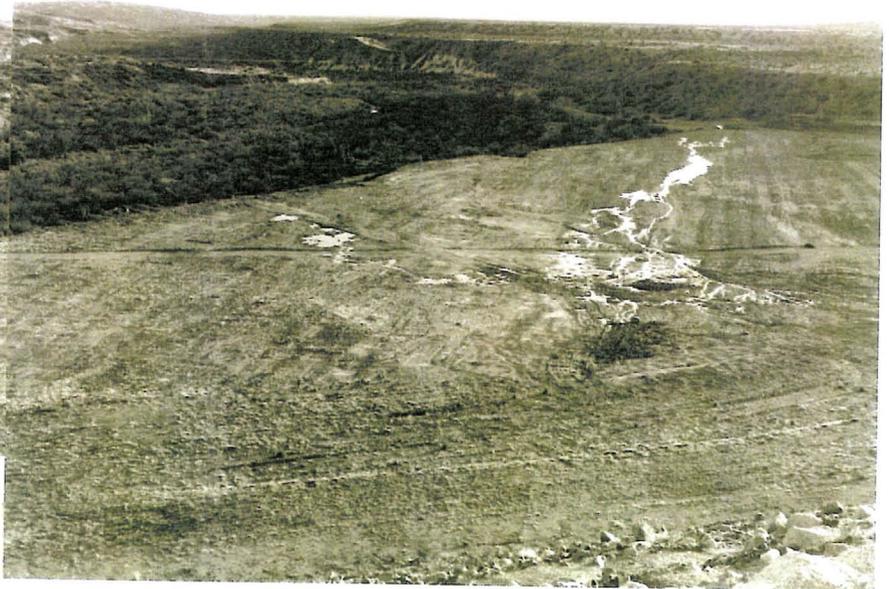


2





APR • 65



APR • 65

TELEPHONE 297-2691

J. EMERY BARKER

ATTORNEY AT LAW

7107 NORTH ORACLE ROAD 181920212

CASAS ADOBES PLAZA

TUCSON, ARIZONA

*Carl W. Barker*



Walt:

Thank you for  
the clipping on the flood  
for Walnut Gulch --  
maybe we ought to quote  
the people at Ahoaca on  
blowing up Golder Dam --  
I think we could sacrifice  
for about 2 million bucks.  
Emery.



# HOFFMAN-MILLER ENGINEERS, INC.

Investigations — Reports — Design — Supervision

RALPH A. HOFFMAN  
LLOYD W. MILLER  
A. KNOX MATTER

1110 EAST MCDOWELL  
P.O. BOX 5363  
PHOENIX, ARIZONA 85010  
PHONES: AL 4-9569 — AL 4-7791

BRIDGES  
HIGHWAYS  
DAMS  
FLOOD CONTROL  
FOUNDATIONS  
DRILLING  
PIPE LINES  
SURVEYS

May 5, 1965

J. Emery Barker, Attorney at Law  
7107 North Oracle Road  
Casas Adobes Plaza  
Tucson, Arizona



Dear Mr. Barker:

Re: Bench Mark Elevations  
Golder Dam

After talking to you personally yesterday, I met Dr. George Kiersch at the Dam Site. He suggested that I mail the elevation information on Bench Marks in the area of the Dam directly to you; that he would then transmit it to the survey party which is providing him with planimetric and vertical control for work he is doing.

The Bench Marks for which the following elevations are given were still prominent yesterday and did not appear to have been disturbed in any way since they were originally set.

DESCRIPTION OF BENCH MARK	CONSTR. DATUM ELEVATION	USGS DATUM ELEVATION
Top of 1 1/2" Iron Pipe @ Sta. 0+00 near West end of Dam	224.76	3408.76
Top of 3/4" Iron Pipe Ref. Pt. to P.C. 200' South of Construction Centerline - Sta. 10+27.0	170.63	3354.63
Top of 3/4" Iron Pipe Ref. Pt. to P.C. 400' South of Construction Centerline - Sta. 10+27.0	167.50	3351.50
Brass Cap set in top of concrete wingwall @ S.W. Corner of Spillway mouth	223.79	3407.79

*Superior Manitowish*

J. Emery Barker, Attorney at Law  
May 5, 1965  
Page 2

Three copies of this transmittal are enclosed and we ask that you deliver one copy to Heinrichs Geocex in the event you feel they have need for it.

Yours very truly,

HOFFMAN-MILLER ENGINEERS, INC.

*A. K. Matter*  
A. K. MATTER

AKM:el



RALPH A. HOFFMAN  
LLOYD W. MILLER  
A. KNOX MATTER

# HOFFMAN-MILLER ENGINEERS, INC.

*Investigations — Reports — Design — Supervision*

1110 72 1242 EAST MCDOWELL  
P.O. BOX 5363  
PHOENIX, ARIZONA 85010

PHONES: AL 4-9569 — AL 4-7791

May 5, 1965

BRIDGES  
HIGHWAYS  
DAMS  
FLOOD CONTROL  
FOUNDATIONS  
DRILLING  
PIPE LINES  
SURVEYS

J. Emery Barker, Attorney at Law  
7107 North Oracle Road  
Casas Adobes Plaza  
Tucson, Arizona

Dear Mr. Barker:

Re: Bench Mark Elevations  
Golder Dam

After talking to you personally yesterday, I met Dr. George Kiersch at the Dam Site. He suggested that I mail the elevation information on Bench Marks in the area of the Dam directly to you; that he would then transmit it to the survey party which is providing him with planimetric and vertical control for work he is doing.

The Bench Marks for which the following elevations are given were still prominent yesterday and did not appear to have been disturbed in any way since they were originally set.

DESCRIPTION OF BENCH MARK	CONSTR. DATUM ELEVATION	USGS DATUM ELEVATION
Top of 1½" Iron Pipe @ Sta. 0+00 near West end of Dam	224.76	3408.76
Top of ¾" Iron Pipe Ref. Pt. to P.C. 200' South of Construction Centerline - Sta. 10+27.0	170.63	3354.63
Top of ¾" Iron Pipe Ref. Pt. to P.C. 400' South of Construction Centerline - Sta. 10+27.0	167.50	3351.50
Brass Cap set in top of concrete wingwall @ S.W. Corner of Spillway mouth	223.79	3407.79

*Release*  
*Superior Manifold*

J. Emery Barker, Attorney at Law  
May 5, 1965  
Page 2

Three copies of this transmittal are enclosed and we ask that you deliver one copy to Heinrichs Geoex in the event you feel they have need for it.

Yours very truly,

HOFFMAN-MILLER ENGINEERS, INC.

*A. K. Matter*  
A. K. MATTER

AKM:el

*Black*

*Superior Manifold*

CREWS WORK AROUND CLOCK

*Rain No Rain*

**Golder Dam Holding Its  
Liquid Better Nowadays**

i  
n  
b  
c  
w  
ti



McGraw Hill  
Ithaca, N.Y.

April 3, 1965.

\*\*\*\*\*  
\*\*\*\*\*  
\*\*\*\*\*

THE TENTATIVE LAYOUT OF OFFICE COMPILATION DISCUSSED WITH

WALTER HEINRICHS, Geos exploration Co.

I. PLAN MAP of Reservoir and Downstream area.

Scale 1" = 100 ft. Approx. Limits shown on original drawing

Topo at C.I. 10 ft. from Drawing 2 of 12  
40 ft. from " 1 of 12  
80 ft. " USGS base

CONTOUR INT

Downstream limit to include large bluff of "Old Gila congl.

II. ON A COPY OF PLAN MAP ( Sheet No. 1 )

Plot the following types of information:

A. Geologic data—

From files, other individuals, Prof. Lacy, et al.  
Includes, area of rock types, fault contacts, et al.

Contact

Fault

B. Hydrologic data

Area of concentrated losses (H-M, 17, Feb. '65, Plate I)  
Leakage

Toe of dam

Borrow Area No. 3

C. Location of cross-sections, A-A, B-B, C-C, and Centerline Dam

D. Core holes, collars with no's

E. Outline of dam (Dashed-in and de-emphasize) (long fine lines)  
Concrete core trench ends about sta. 17+00 (Omit spillway)

III. Cross Sections GEOLOGIC - True Scale HW.

Plot on each from available data

Surface and subsurface rock information, Drill holes

Water levels

Pre-construction

Post-construction and when reservoir impounding water

SECTION A-A, Located Downstream of dam, scale of 1" = 100 ft.

" B-B, Upstream of Dam, scale 1" = 100 ft.

" C-C, Right Bank area to Turning Point (more possibly later), scale 1" = 100 ft.

DAM AXIS (original section) re-plot to True scale, HW (Sta 0+00 to 29+20)  
Drawing A of 12

Drill Hole data plotted

Any interpretation of Geophysical data deemed pertinent for deep parts

George A. Kiarock

Exec Copy

McCraw Hall  
Ithaca, N.Y.

April 3, 1965.

\*\*\*\*\*  
\*\*\*\*\*  
\*\*\*\*\*

THE TENTATIVE LAYOUT OF OFFICE CORRELATION DISCUSSED WITH

WALTER HEINRICHS, Geos exploration Co.

APPX 66" N-S  
- EW

I. PLAN MAP of Reservoir and Downstream area.  
Scale 1" = 100 ft. Approx. Limits shown on original drawing

VARIABLE

Topo at C.I. 10 ft. from Drawing 2 of 12  
40 ft. from " 1 of 12  
80 ft. " USGS base  
Downstream limit to include large bluff of "Old Gila cong.?"

OK

II. ON A COPY OF PLAN MAP (Sheet No. 1)  
Plot the following types of information:

Contact Fault

A. Geologic data—  
From files, other individuals, Prof. Lacy, et al.  
Includes, area of rock types, fault contacts, et al.

BLUE H.M. REPORT

B. Hydrologic data  
Area of concentrated losses (H-M, 17, Feb. '65, Plate I)  
Leakage  
Toe of dam  
Borrow Area No. 3

PLAN MAP

C. Location of cross-sections, A-A, B-B, C-C, and Centerline Dam

H.M. ENG. TEST  
BORING LOC.

D. Core holes, collars with NO's

E. Outline of dam (Dashed-in and de-emphasize) (long fine lines)  
Concrete core trench ends about sta. 17+00 (Omit spillway)

III. Cross Sections GEOLOGIC - True Scale H&V.

Plot on each from available data  
Surface and subsurface rock information, Drill holes  
Water levels  
Pre-construction  
Post-construction and when reservoir impounding water

SECTION A-A, Located Downstream of dam, scale of 1" = 100 ft.  
" B-B, Upstream of Dam, scale 1" = 100 ft.  
" C-C, Right Bank area to Turning Point (more possibly later), scale 1" = 100 ft.

DAM AXIS (original section) red to True scale, H&V (Sta 0+00 to 29+20)  
Drawing A of 12

(Drill Hole data plotted  
Any interpretation of Geophysical data deemed pertinent for deep parts

George A. Kiersch

3900  
215  

---

3185

14.00  
.6  
13.40

215  

---

205 - 10  
195 - 20  
185 - 30  

---

175 - 40  
165  

---

155  
145  

---

135  
125  

---

115  
105  

---

95  
85  

---

75  
65  

---

55  
45  

---

35  
25  

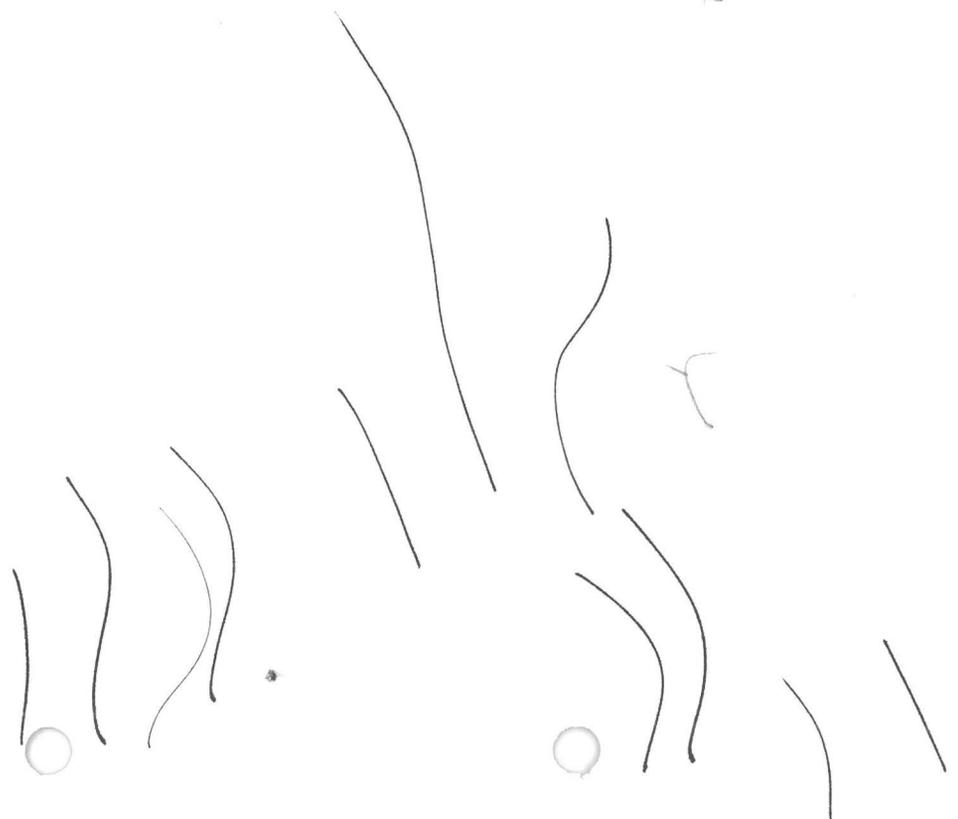
---

15  
5

D

L

A



Paul W. Rowell  
Holder

Samuel F. Turner & Associates report

French Drain plan Hoffman-Miller

Minutes of Meeting 9/3/63

Materials Investigation - Golden Dam Southwest Materials Lab -

Letter - Hoffman et al 12/2/64

" " 10/23/64

" " 11/2/64

Test Boring Logs -

Wheeler Peterson Tests -

Report of Ariz Testing Labs

Letter 11/11/64

Report on Golden Dam Turner - 10/60

Report Hoffman-Miller 2/17/65

E. R. Geophysical Report 4/29/63

12 Pictures 8x10's

Copy of PLANS

Received of  
HENRICHS GEOEX

9/10/65  
Erney Baker



## HEINRICHS GEOEXPLORATION COMPANY

806 WEST GRANT ROAD, TUCSON, ARIZONA, 85708. P.O. BOX 5671. PHONE: (AREA CODE 602) 623-0578

April 7, 1965

Rail N Ranch Corporation  
7107 N. Oracle Road  
Casas Adobes Plaza  
Tucson, Arizona

Re: Proposed Golder Dam Investigations

Attn: Mr. J. Emery Barker, Attorney

Gentlemen:

During the past several months we have had an occasional phone conversation with Dr. Willard Lacy regarding above subject. On the 29th of last month, Mr. Barker called and an appointment was made and a brief conference was held on March 31, 1965 with Mr. Barker and Mr. George Kiersh and myself in attendance. Later on that same day, Mr. Kiersh and I made a brief field inspection visit to the site. Following this, several phone conversations with Mr. Kiersh ensued and on April 3, 1965, the whole day was spent with Mr. Kiersh in a comprehensive review of all available data and planning the detailed steps and recommendations for phase one of the investigation. Three others of the GEOEX staff were also briefly in attendance. This work resulted in a tentative lay out of office compilation which was set forth in a letter to you from Mr. Kiersh of the same date.

In conformance with all of the above and based on our verbal estimates of time and cost involved it is our understanding that you wish to have us proceed with this phase of the investigation and accordingly this is now under way. Our fees for this work will be at the rate of \$100.00 per staff day plus expenses, which will be mainly for reproductions, photographs, etc., and will require approximately ten days to complete. Fee for my professional time involved will be at the rate of \$150.00 per day. In addition, it is understood and agreed that we will confer with Dr. Lacy for which he will be compensated for this and past services rendered in the amount of \$500.00. Summarizing the above, in using a figure of \$500.00 as a total for expenses

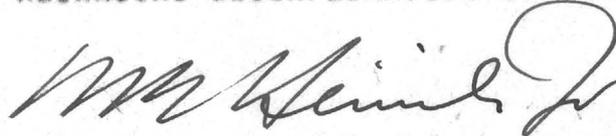
April 7, 1965

and contingency factor, we estimate a maximum expenditure in the neighborhood of \$2,500.00 for this work.

For our mutual convenience if this meets with your approval, your acceptance of same may be indicated by executing in the spaces provided below and returning the extra carbon copy to this office.

Billings are on a bi-weekly basis or as time and expenses are accrued. In view of anticipated outlays of funds to Dr. Lacy and those already accrued in the form of compilation and reproductions, expenses, etc., we request as a binder on continuing to proceed with the work, the sum of \$1,000.00.

Very truly yours,  
HEINRICHS GEOEXPLORATION COMPANY



Walter E. Heinrichs, Jr.  
President & General Manager

WEH:jh

cc: J. E. Barker  
George Kiersh

Approved: \_\_\_\_\_  
Rail N. Ranch Corporation

Date: \_\_\_\_\_

Title: \_\_\_\_\_

HOFFMAN-MILLER ENGINEERS, INC.  
Investigations--Reports--Design--Supervision  
1242 East McDowell Road  
P.O. Box 5363  
Phoenix 10, Arizona  
Phones: AL 4-9569--AL 4-7791

Copy <sup>LOG</sup>  
Ro

ENGINEERS REPORT  
ON  
GOLDER DAM

LOCATION: The proposed dam will impound the waters of the Canada del Oro in an area along the East half of sections 13 and 24 of Township 10 South and Range 14 East in Pinal County, Arizona. The dam site is situated approximately 5 miles due East of Oracle Junction, Arizona, in the SE $\frac{1}{4}$  of Section 24, Twp. 10 S, R 14 E, Gila and Salt River Base & Meridian.

OWNER: The owner of the project is the Rail N Ranch Corporation. The project will be built in connection with a housing development to provide a recreational area for the project. The reservoir at its maximum capacity will provide an area of 260 acres having a total length of about 1-3/4 miles and a width of approximately 1/2 mile at its widest point.

DRAINAGE AREA: The drainage area behind the dam, as calculated from the most recent U.S.G.S. maps, amounts to 48 sq. miles ranging in elevation from 3400 ft. at the dam to 9000 ft. on the North slope of Mt. Lemmon.

The Canada del Oro flows in a Northerly direction from the higher timbered slopes of the mountain to a point about 4 miles South of the Town of Oracle, Arizona, where the stream makes a 180° bend and flows South to the proposed dam site; thence in a South and Southwesterly direction to a junction with the Santa Cruz River.

RAINFALL AND RUNOFF: Rainfall and runoff records are not available; however, climatological data and previous studies of the area indicate that the average yearly rainfall will amount to 15 inches to 20 inches. By comparison with records of similar streams in the area it is anticipated that the average annual yield at the dam site will amount to 7,500 to 10,000 Acre Feet. This amount of water will be sufficient to maintain the proposed lake which will have a maximum storage capacity of 9,500 Acre Feet.

An inspection of the streambed at the site and for several miles upstream from the site, indicates that flood flows of recent history have been relatively small and the marks left by these floods are of no value in determining maximum flood conditions. Since there are no gauging records available for the stream, the design of the spillway must be resolved by using basic calculations for the area.

SPILLWAY DESIGN: The design capacity of the spillway was determined by using a basic 6 hour storm with a rainfall of 5 inches over the 48 sq. mile area. As shown by the calculations the maximum condition was produced by a 5 hour storm with a total rainfall of 4.75 inches and an equivalent runoff of 2.25 inches (47.5%) producing a maximum inflow to the lake of 14,240 cfs.

The volume of surcharge storage provided by the 6 ft. above the spillway crest is estimated at 1800 Acre Ft. which is equivalent to 0.7 inch of runoff over the entire drainage area. This surcharge cushion reduces the maximum flow at the crest to 10,440 cfs which is the basis for the spillway design giving a 5 ft. freeboard on the dam.

The spillway will be cut through the granite rock that is exposed on the East abutment of the dam.

The spillway channel below the crest has been so designed as to provide control of maximum flood conditions on the 230 ft. spillway crest.

The spillway will discharge into a natural drainage channel re-entering the Canada del Oro channel at a point just below an old rock and masonry dam well below the toe of the new dam.

FOUNDATIONS: The slopes to the East of the main channel are, in general, exposed granite rock. The slopes to the West of the channel, except for a small area adjacent to the channel, consists of an alluvium fill underlain by the granite at a varying depth.

The site was drilled using diamond drilling in the solid rock areas and auger and wash boring methods on the alluvium areas. Results of these borings are indicated on Sheet #1A of the plans. The design plans contemplate a concrete cut-off

wall on the exposed rock area and thru the main channel. Grouting is provided thru this wall although the drilling tests indicate that the rock will take very little grout.

A core trench is provided in the alluvium areas to various depths as indicated anticipating that the clays and soft decayed layer of rock will provide an adequate seal in these areas.

MATERIALS: Extensive prospecting for materials was done in the vicinity of the dam and laboratory test made to determine the quality of the material for the various zones of construction.

The location of test pits and borrow areas are shown on Sheet #1 of the plans. The logs of the pits and tests of samples are given in the materials investigation report accompanying this submission.

DESIGN: The design of the dam is based on an impervious section, Zone C, comprising the main body of the dam and buttressed by pervious zones of sand, gravel, and rock.

The impervious zone will be composed of a mixture of decomposed granite and clays containing rock fragments. The sources of these materials and the manipulation are set forth in the specifications.

The pervious zones are composed of sand, gravel and rock (cobbles). Since there is a large percentage of large rock (cobbles) in some areas of the borrow, it is anticipated that the specified spreading and raking of this material will provide an adequate blanket of rock on both faces of the dam so as to provide adequate slope protection. A minimum of 2 ft. blanket is specified for the front face with the provision that rock from rock excavations be reserved to supplement the material for this blanket where required.

OUTLET WORKS: The outlet works is designed to provide a measure of safety during construction along with a means of controlling the water surface after completion.

A gate tower is provided with two gates; one, a 42" slide gate for the purpose of the control of water behind the cofferdam during construction, and a 24" hydraulically operated

double disc gate valve as a permanent installation to afford a measure of lake level control when desired.

A 36 inch welded pipe encased in concrete is provided for the outlet through the dam. This size gave the most efficient control of water during construction at a reasonable cost to the project.

FLOOD CONTROL DURING CONSTRUCTION: The outlet works provides a means for the Contractor to pass flood waters during the construction period. A minimum freeboard of 10 feet is required on the upstream construction cofferdam. This 10 feet of freeboard has a volume of 169 acre feet. The outlet works will pass approximately 160 acre feet of water per day under the above conditions.

As the construction of the embankment of the dam rises above the top of the cofferdam, additional freeboard and storage capacity are available. When the embankment reaches an elevation of 177, the 24 inch gate valve may be installed and the stored water raised to elevation 137. This 40 feet of freeboard represents 2,400 acre feet of storage which is equivalent to 0.94 inches of runoff (3 inch rain) over the entire drainage area. The outlet works, with the 24 inch valve, will pass 160 acre feet of water per day with the lake level at Elevation 137 and will increase in capacity to 210 acre feet of water per day with the lake level at Elevation 177.

By maintaining a minimum freeboard of 40 feet, the storage capacity greatly increases as the embankment is built. The outlet works has a capacity to pass 5,000 to 6,000 acre feet of water per month, which will amply provide for the anticipated flows in the Canada del Oro.

It is realized that a flood hazard does exist during the initial stages of construction; however, the amount of impounded water at this time is of such a small nature that damages would be very small. The distance from the dam site to the bridge on State Highway 80-89 is ten miles. This reach of the Canada del Oro is uninhabited and undeveloped and any flood increase due to the small storage behind the cofferdam would dissipate itself within this area.

ENGINEERS ESTIMATE: The Engineers estimate indicated below is based on the basis for measurement and payment set forth in the Specifications. Payment will be made on all quantities within the slope stakes for both excavation and embankment. This method does result in double payment for some items of excavation placed in embankment zones, but should result in a substantial reduction in the prices of excavation and alleviate any confusion as to the adjustment of pay quantities for various zones of embankment.

ENGINEERS COST ESTIMATE

<u>Item</u>	<u>quantities</u>	<u>Description</u>	<u>Unit Price</u>	<u>Total</u>
1	One LS	Clearing & Grubbing	2,000.00	2,000.00
2	85,000 CY	Spillway Excav.	.85	72,250.00
3	80,000 CY	Common Excavation	.45	36,000.00
4.	755,000 CY	Embank.-Imperv. Zones	.45	339,750.00
5	510,000 CY	Embank.-Perv. Zones	.70	357,000.00
6	34,400 MG	Sluicing-Perv. Zones	.40	13,760.00
7	910 CY	Class A Concrete	60.00	54,600.00
8	51,500 LB	Reinforcing Steel	.15	7,725.00
9	124 LF	42" Corr. Met. In. Pipe	25.00	3,100.00
10	575 LF	36 Sm. Steel Out. Pipe	25.00	14,375.00
11	One LS	Inlet Gr. & Manh. Cover	800.00	800.00
12	One LS	24" Gate & Appurtenances	6,000.00	6,000.00
13	One LS	42" Cons. Slide Gate Valve	3,500.00	3,500.00
14	1,500 LF	Groute Pipe	1.00	1,500.00
15	4,500 LF	Drilling Grout Holes	1.50	6,750.00
16	2,000 Sks	Filling Grout Holes	3.00	6,000.00
17	2,370 LF	6" Perforated Corr. Met. Pipe	3.00	7,110.00
				\$932,220.00
Engineering & Contingencies				62,780.00
				\$995,000.00

# ALLIED RADIO CORPORATION

of California

2085 E. FOOTHILL BOULEVARD · PASADENA, CALIFORNIA

(Area Code 213) 795-5901 · 684-0730 · TWX 213-449-1455

---

PLOT proposed Drain  
HOLE Sec A-A' OK

---

PLOT GR on  
PLAN MAT. ✓ OK

---

PROJECT on Sec.  
A-A'

---

B B'

AXIS

A A'



TEXAS INSTRUMENTS  
INCORPORATED

CHANGE ELEV. ON  
DAM AXIS OF OK  
PROFILES

---

WATER TABLE ELEV ?

---

BILL LACY  
MCCULLOUGH  
ED - ~~DAVISON~~ GEOLOGIST AT  
U.A.

DOUG LEWIS

BARKER = WATER TABLE  
LEVEL AT WELL -

---

624 8181 - 285

43  
3241

3400

H. 1 ~~5~~ = 5 FT

MR. PIERSON  
SANDY  
BERECH

1. FIND A CONTROL POINT ON GROUND  
2. PLOT ON MAP.

3. FIND LOC. OF ISLAND & SPRING (TREE)

~~4.~~ 3. <sup>check</sup> location K1 drill hole

4. Check w/ diller.

PIN DRILL

38' - 27 April

5. Elev. reservoir.

4:50

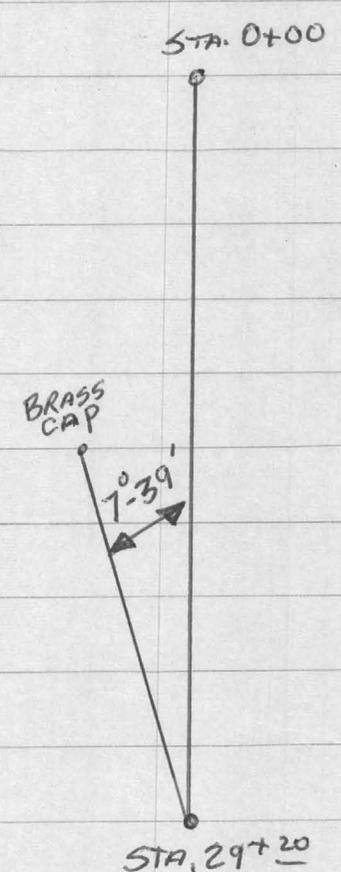
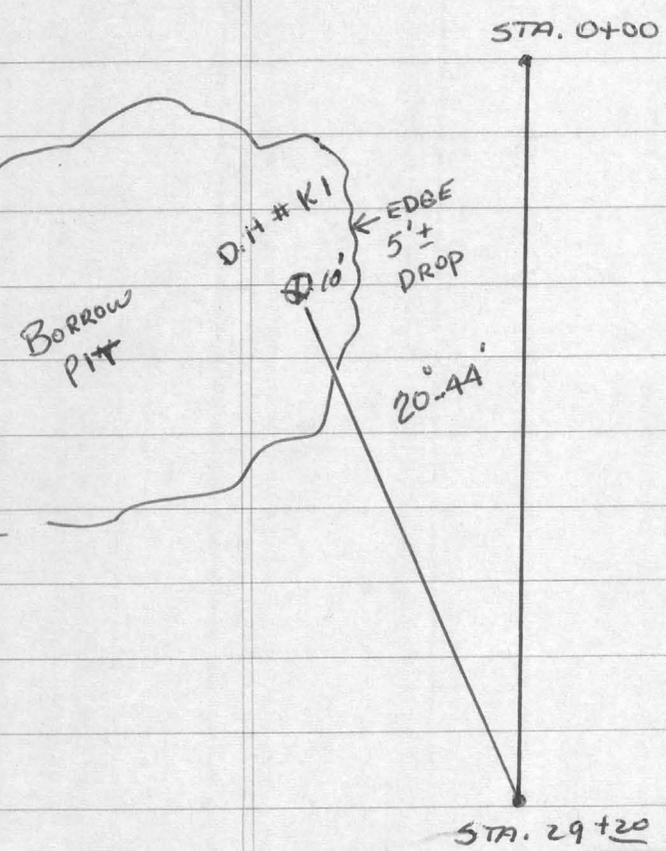
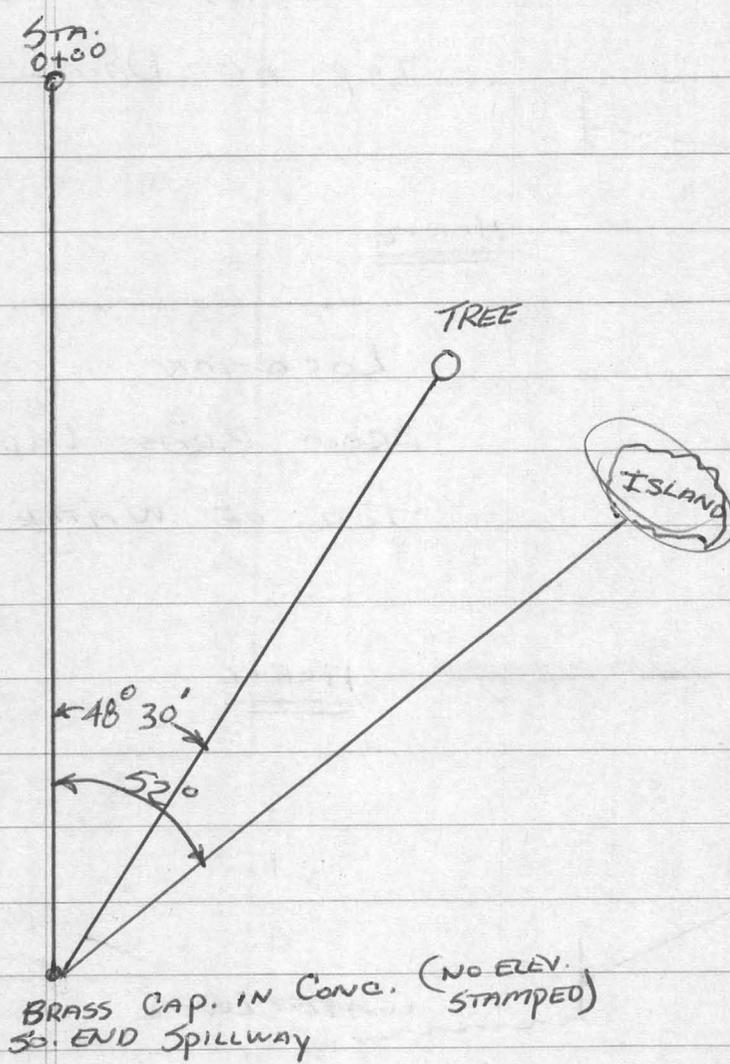
So. End Spillway (BRASS CAP.)

NEAR CONTROL BOX

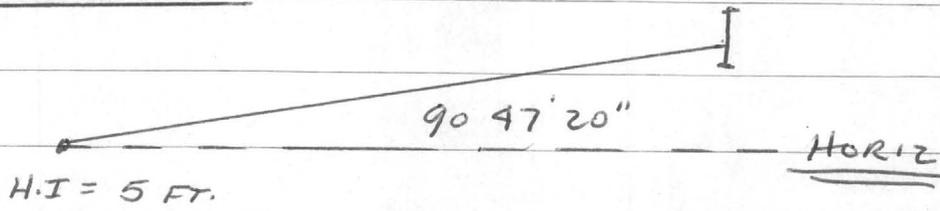
2/2/2014

# GOLDE DAM

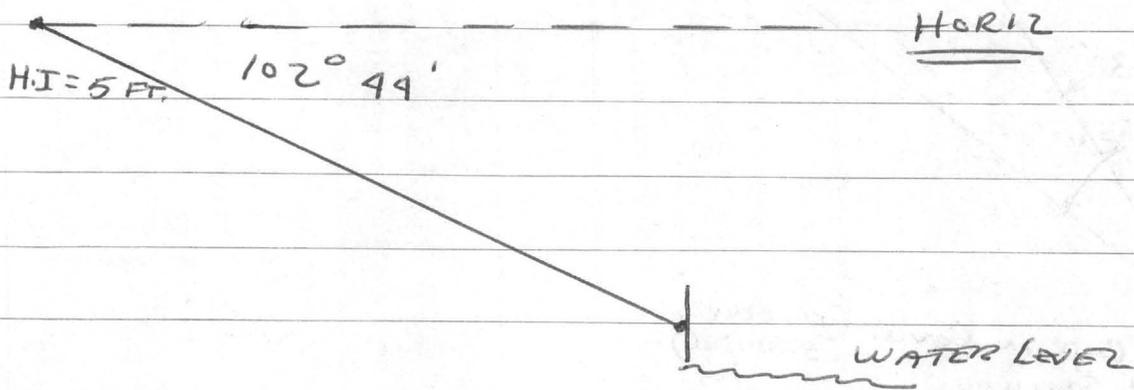
MOULINET  
JOHNSON  
APRIL 28, 1965



Δ	DIST.	LOCATION
$90^{\circ} 47' 20''$	1050	FROM BRASS CAP TO
<u><math>0^{\circ} 47' 20''</math></u>		TOP OF DAM.



Δ	DIST.	LOCATION
$102^{\circ} 44'$	345	FROM BRASS CAP TO
$\frac{90\ 00}{12^{\circ} 44'}$		TOP OF WATER LEVEL





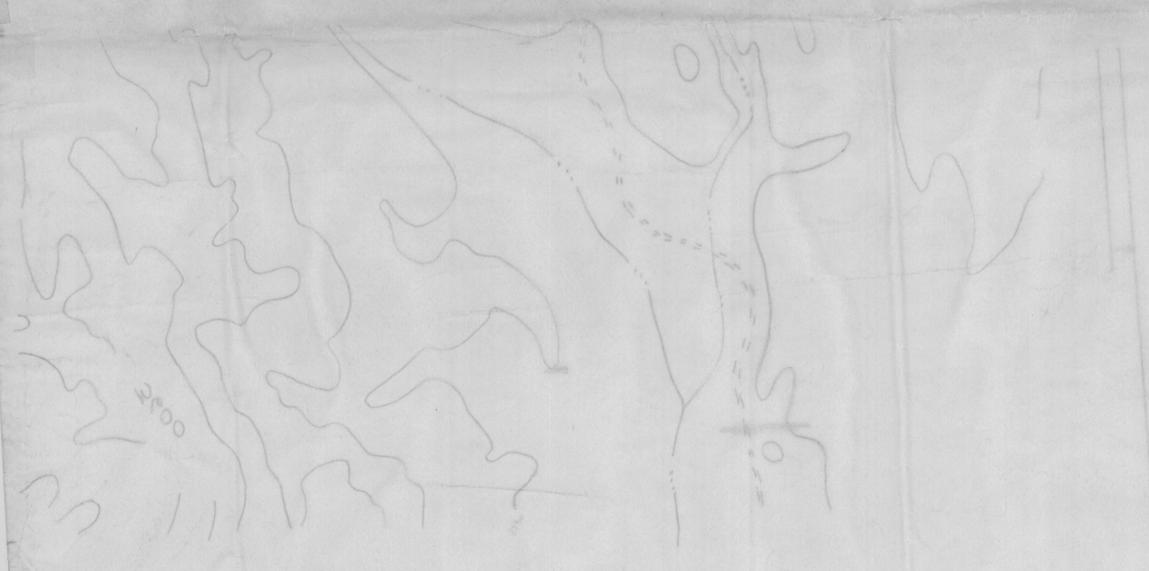
Folder DAM

Golden Ham

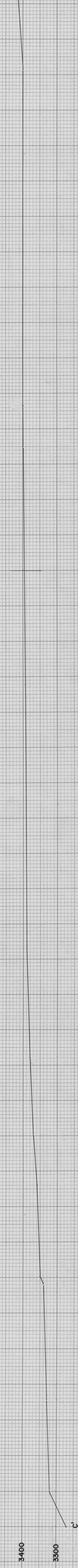
The Capacity Acre Feet
67
202
422
747
212
352
1707
827
267
117
457

INARY  
IN STUDY  
p 20 1982  
dam =  
atum.

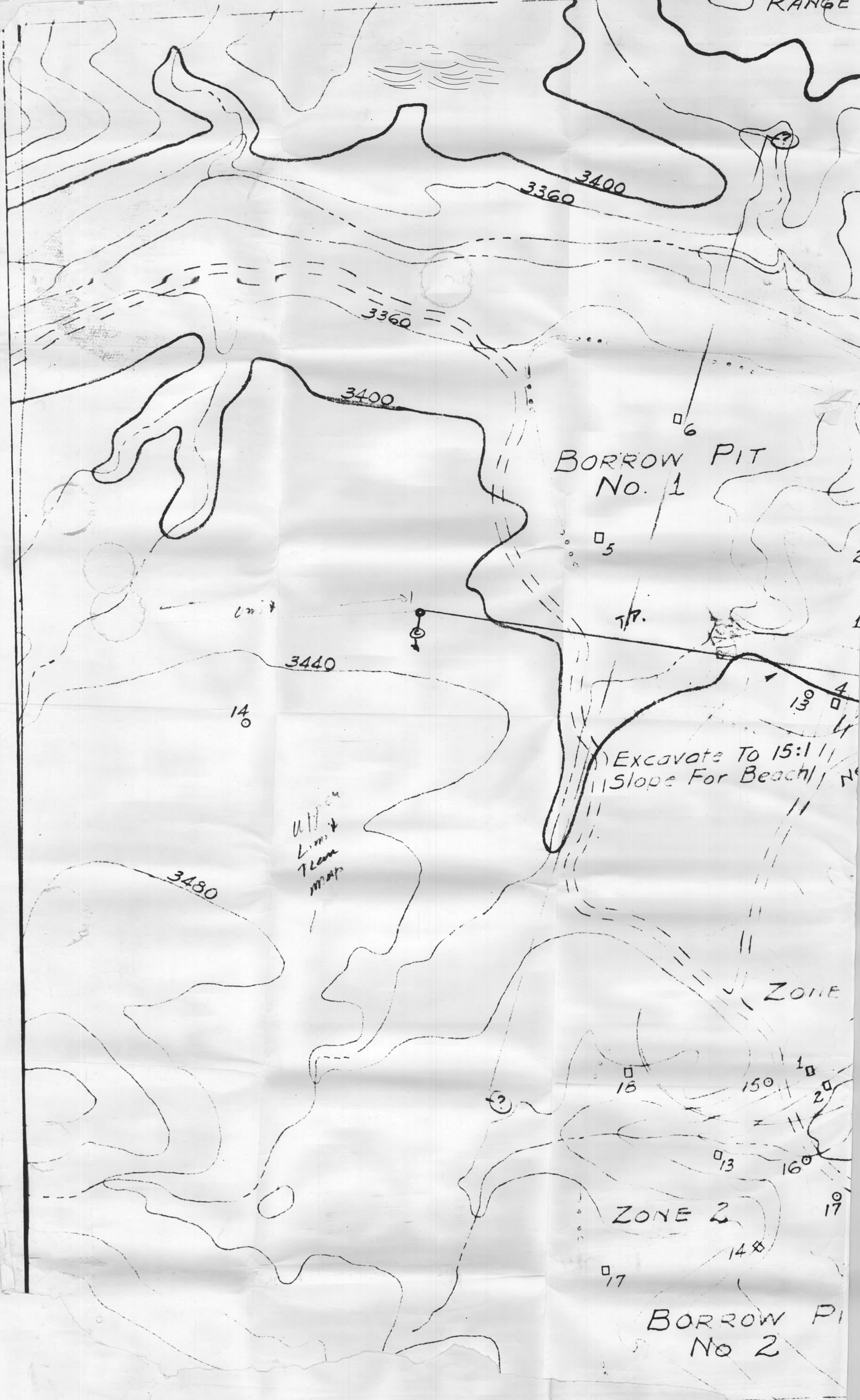
ION  
IONA  
1912



Golden Ham Dam



RANGE  
RANGE



BORROW PIT  
No. 1

Excavate to 15:1  
Slope for beach

ZONE 1

ZONE 2

BORROW PIT  
No. 2

Use on  
Limit &  
Team  
map

Limit

3480

3440

3400

3360

3400  
3360

5

6

14

13

4

4

18

150

10

20

13

16

17

14

17

15-L  
14-E

SEC 24  
3500

Spillway

Inlet Tower

Sta. 16+98.6 P.I.  
 $\Delta = 15^\circ 22' 30''$  Lt.  
(Survey E)

BORROW  
PIT  
No. 3

ew 24' Road

Sta. 0+00  
(Survey E)

Excavate to  
Elev. 3385 ±

