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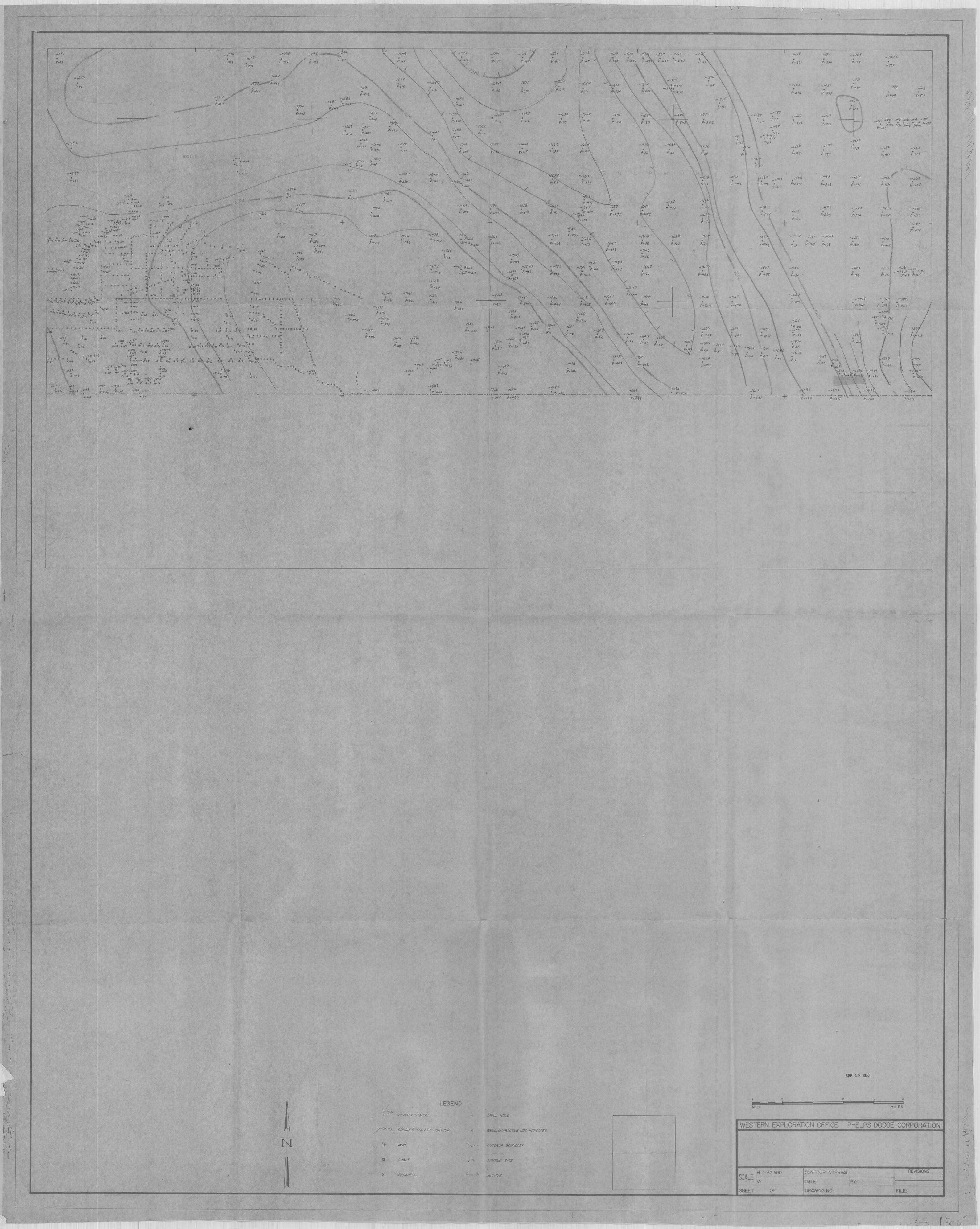
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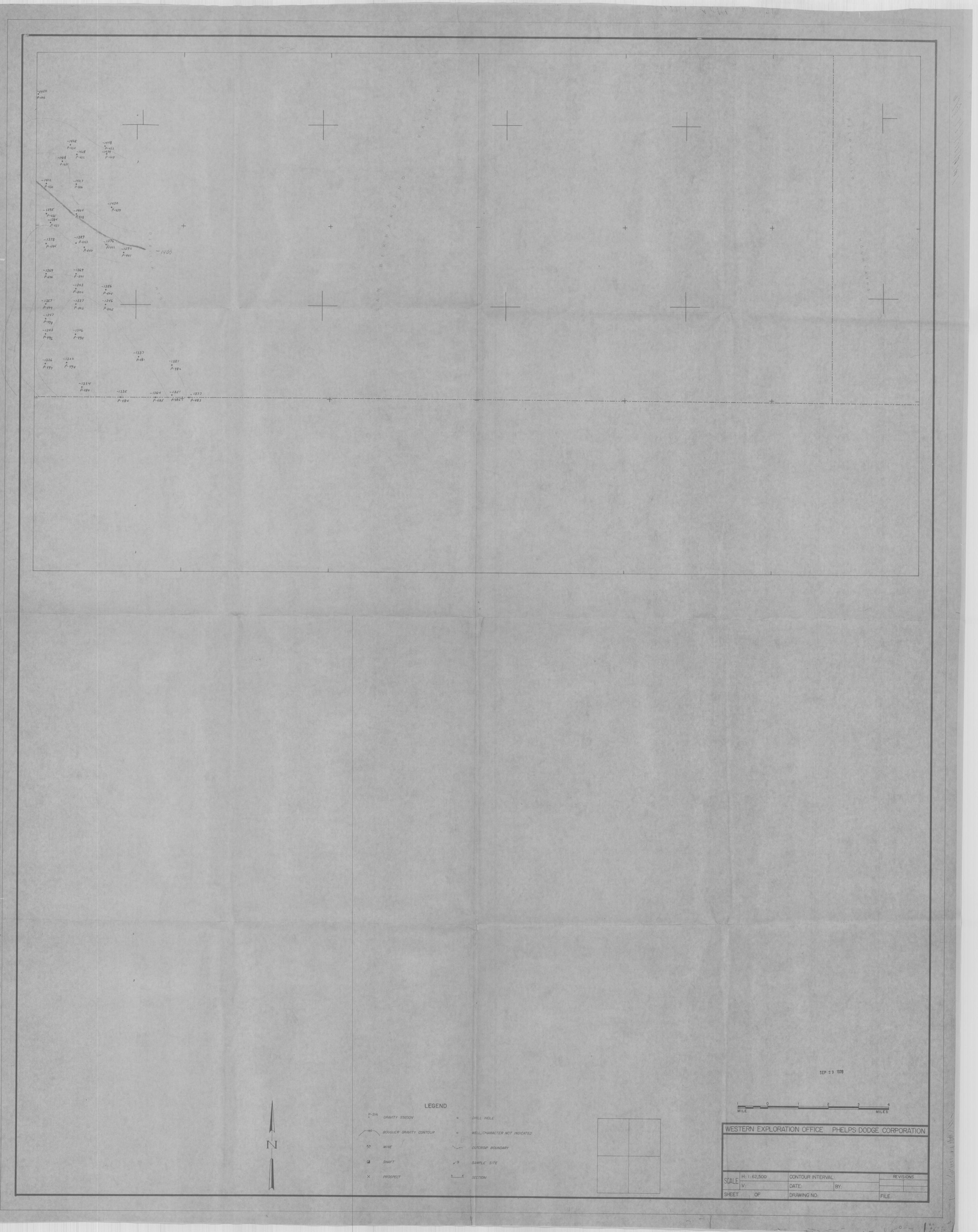
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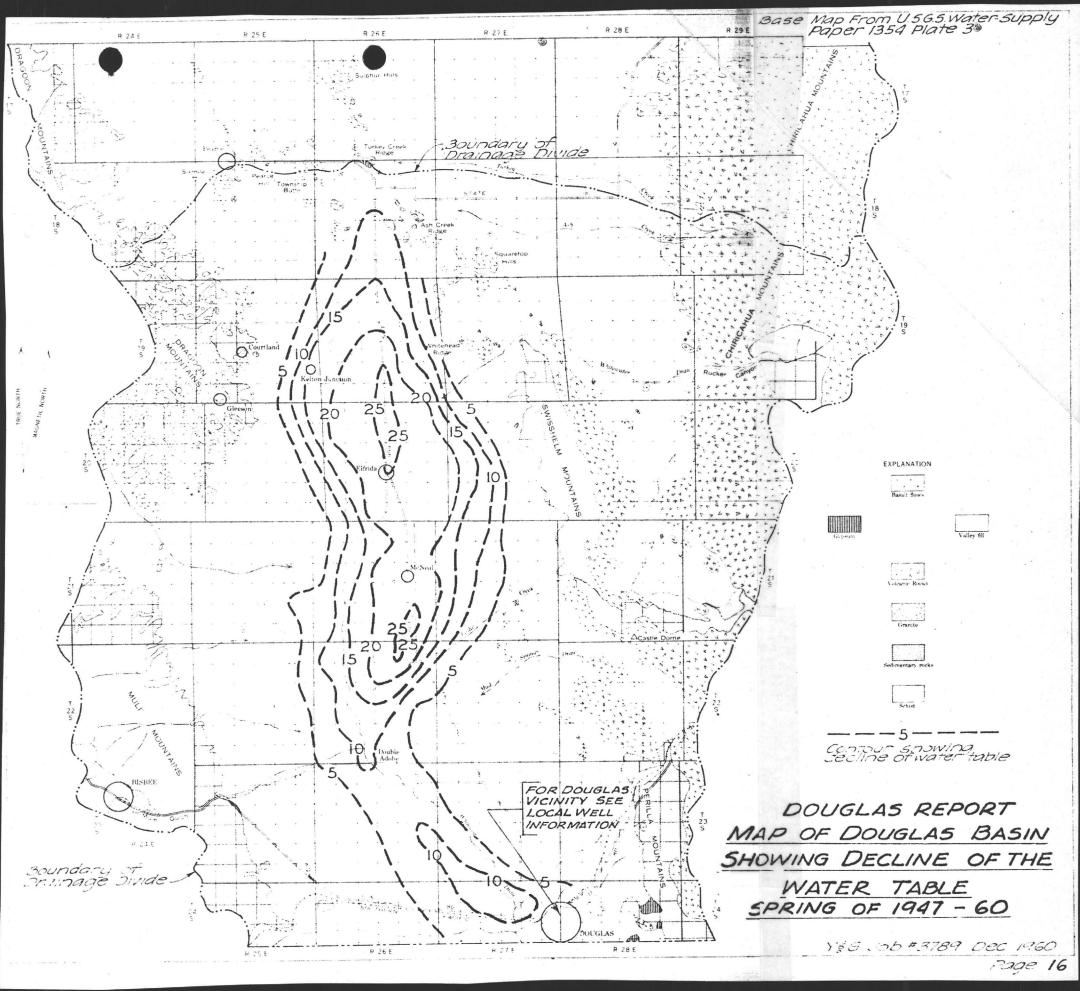
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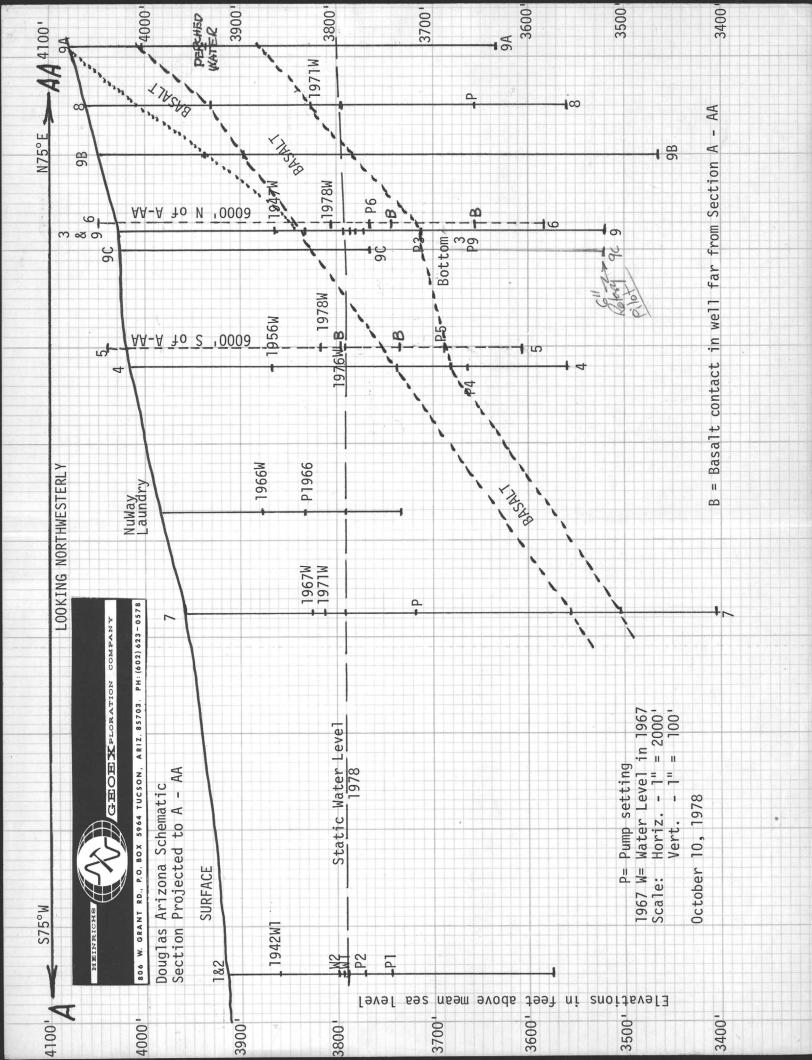
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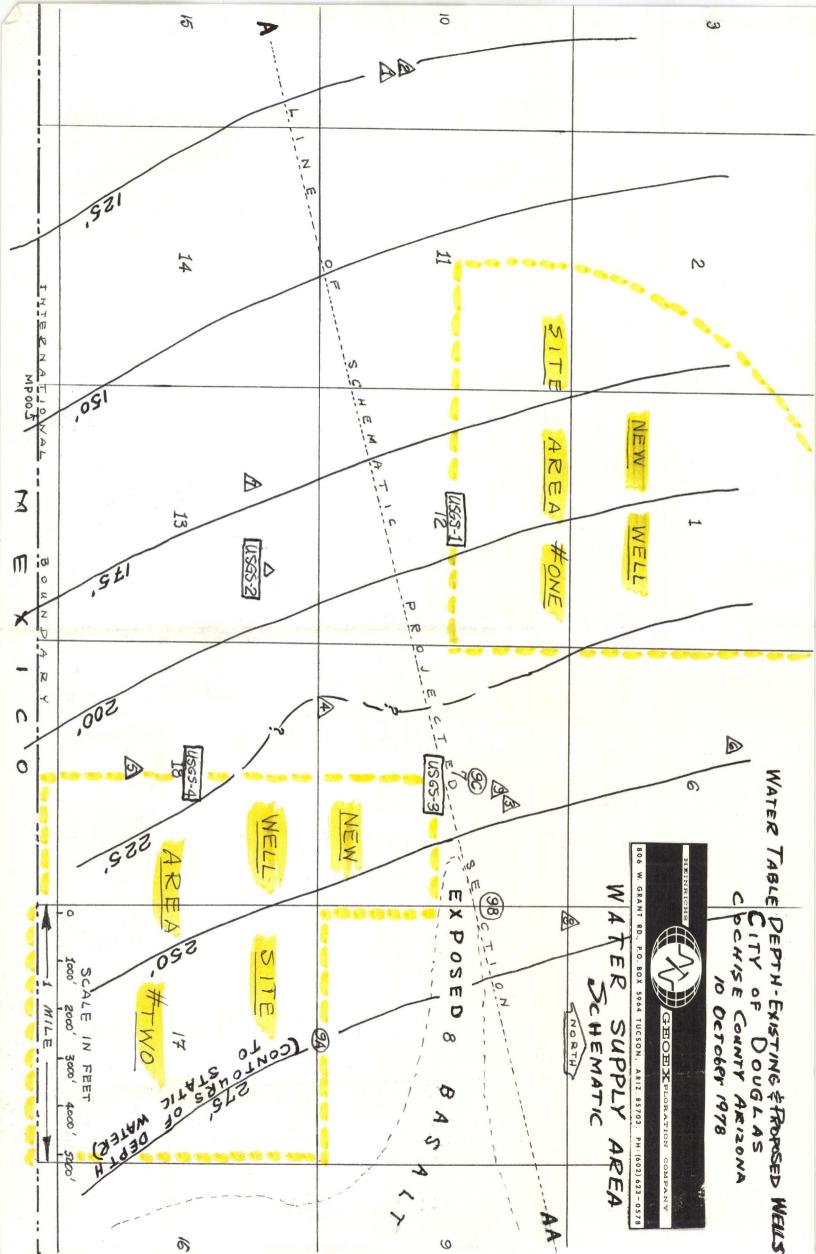






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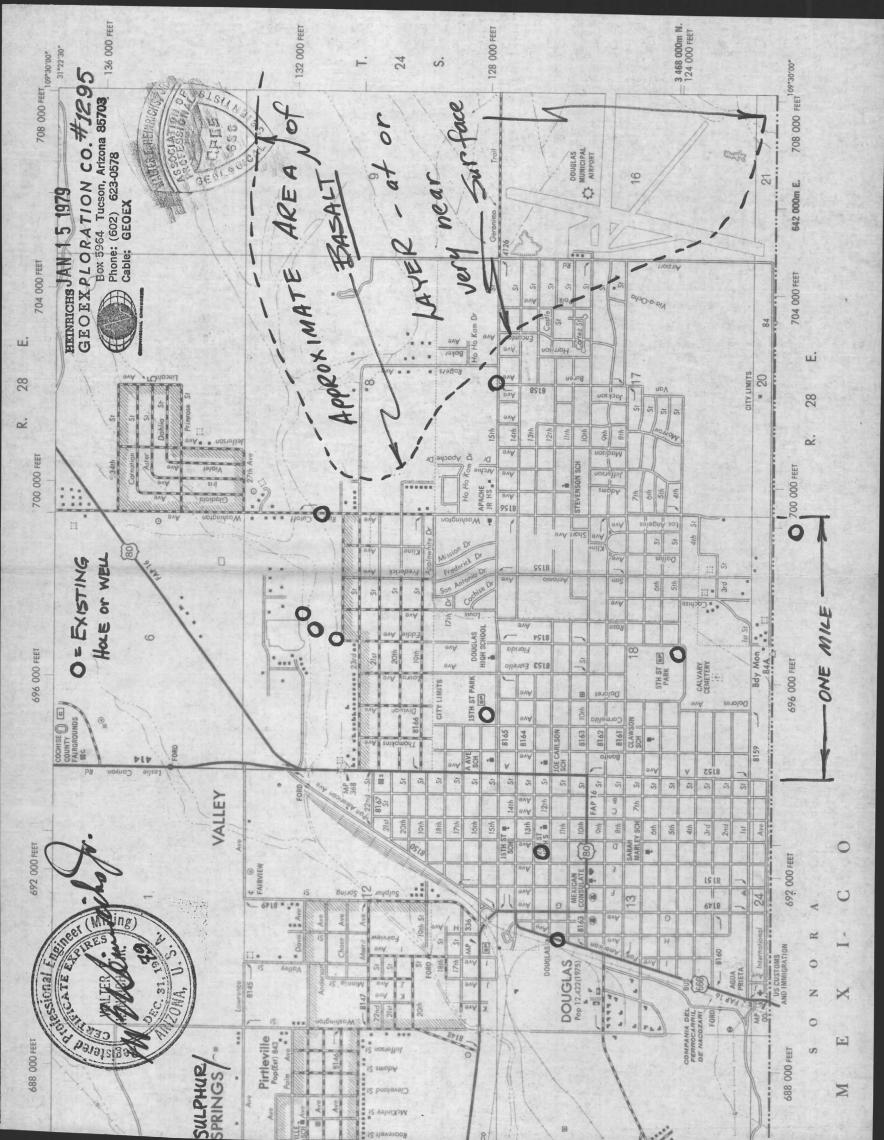


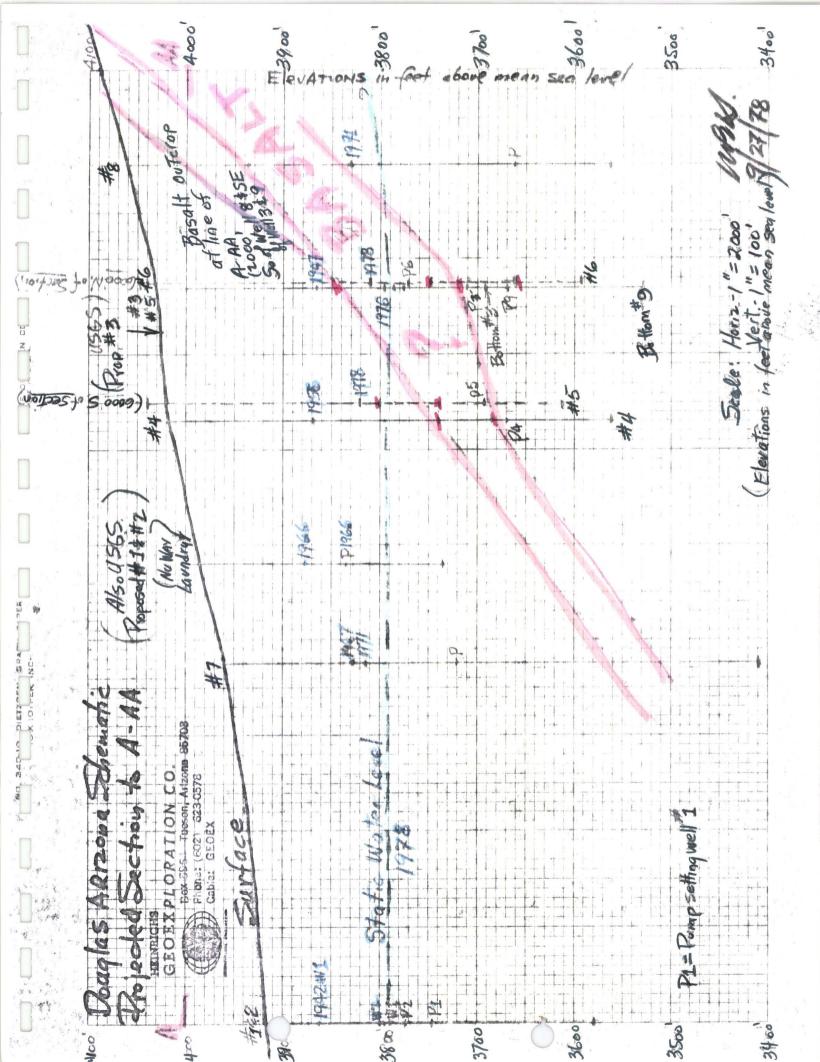


WATER DEPT. 425 - 10th ST. DOUGLAS, AZ. 85607

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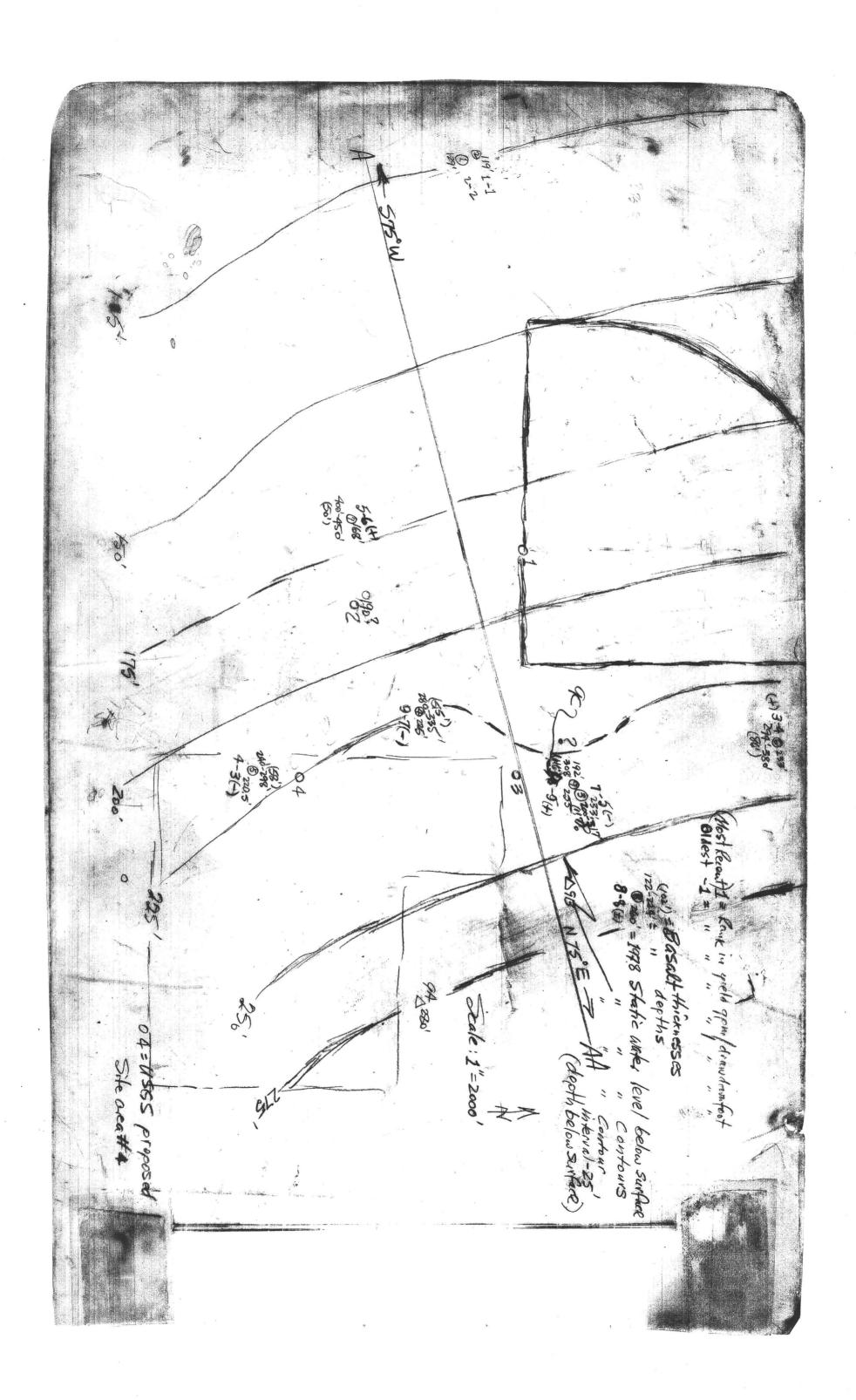
CITY OF DOUGLAS WATER DEPT.





Geo. Nelley Steve Conser Grace Marrill Vic Daniel Louis acosta Bill Merces Joe Cornijo Tersa Scott.

125 0 2000 75 (+) 3.4 @ 253 by 586 (%) w 200 0 8-8(±) = 1978 Static water level 64 280' Teale: 1"= 2000" 59511=40 (depth below surface) Site area# ght Contours



PRELIMINARY GEO-HYDROLOGICAL ANALYSIS

For

THE CITY OF DOUGLAS, ARIZONA

Sept. - Oct. 1978

by

Heinrichs GEOEXploration Company P.O. Box 5964 Tucson, AZ 85703

GEOEX #1295

Preliminary Geo-hydrological Analysis

Introduction

Pursuant to correspondence initiated by Mr. George D. Nalley, Superintendent of the Douglas Water and Sewer Department on 17 July 1978, GEOEX, on 18 September 1978, was engaged to conduct a brief preliminary hydrological survey for the City. Objective of the survey was to assist in the siting of a proposed new water well under consideration for the purpose of augmenting, in the most efficient manner feasible, the present nine-well-system. Data and technical records requested by GEOEX on 19 September, 1978, were received on 22 September 1978. These, together with material already on hand and other pertinent locally acquired information, were studied with results as summarized in this preliminary report.

Conclusions and Recommendations:

- 1. United States Geological Survey (U.S.G.S.) Trip Report of 18 February 1977, copy of which was supplied by the City, tentatively recommended an area (1), proposed to be located near the middle of Section 12, T.24S., R.27E.
 This is concurred to be a satisfactory primary new well site.
- 2. Alternatively, anywhere roughly near or within a pie-shaped wedge area bordered approximately on the south by 22nd Street and Merrit Avenue in Pirtleville, and on the east by A Avenue and Leslie Canyon Road, is considered equally satisfactory except for perhaps distribution and rights of way considerations which are not necessarily part of this immediate study.
- 3. U.S.G.S. proposed areas numbers (3) and (4), are <u>not</u> recommended from the most preferable geologic point of view as suggested by present data on hand.

Based on the city records provided, and although there is some exception and variation indicated, there seems to be a definite general quantity decrease of well yields in an easterly direction away from White Water Draw. Therefore, on that basis alone, it appears generally desirable at this point to keep closer to White Water Draw. Possible exceptions to this generalization include a question of why well 6 is a relatively better producer and why well 4 is relatively poorer. Conceivably, the basalt and the pre-basalt topography and related shallow formations, may be at least partially responsible. If it becomes necessary and important to further pursue these particular types of discrepancies, surface geophysical work, such as electrical resistivity, may warrant testing and could be helpful if tests' results proved favorable. 4. If rights of way and distribution factors outweigh maximum added quantity considerations, then U.S.G.S. area (2) should also be satisfactory and, in some respects may almost equal their area (1). Indicated possible exceptions to this are the proposed site's proximity to poorer well site 4 and its indicated more adverse subsurface conditions, possible interference with well 7, and probable interference with Nu Way Laundry's well if it is still producing and, if so, if that should be a necessary factor to consider.

- <u>5.</u> Existence of "several other test wells" is mentioned in the U.S.G.S. Trip Report of February 1977. Although these are reportedly negative in terms of quantity results, the data still should be integrated with the results of this study because such information could change the conclusions and recommendations herein considerably.
- $\underline{6}$. Long range implications of an anticipated continued increasing rate of ground water level decline and consumptive uses should be kept in mind, both as

to quantity and quality. Ideally, a replenishable surface water system, combined with conservation measures and balanced ground water recharge and withdrawal arrangements should be the ultimate objective of all responsible municipalities. Although beyond the scope of this particular study, problems and possibilities along such lines should be investigated and implemented as rapidly as ways and means will allow.

Observations and Speculation:

- 1. At present the lowest static water table measurement is at an elevation of about 3790 feet near White Water Draw at well #1. This is at about 130 feet below surface. Three and one quarter miles eastward, at Well 8, present static water elevation measures just above the 3800 foot elevation. This represents a rise in static water table elevation of roughly 3 feet per mile from White Water Draw toward the east as far as Well site #8.
- 2. Top of basalt at Well 4 is 40 feet deeper than at well 5, with a total thickness of 55 feet at well 4 and 58 feet at well 5. This is indicative of a possible small northerly dipping component to the basalt layer and perhaps a somewhat southeasterly source, conceivably, even fairly close by. However, as indicated in U.S.G.S. Trip Report of 18 February 1977, specific significance of the basalt relative to local ground water supplies, if any, is relatively undetermined.
- 3. Basalt thickness varies from 50 feet at Well 7 to 117 feet at Well 3. It outcrops about 2000' south of Well 8 and roughly 2000 feet southeast of wells 3 and 9. Average locally measured down-slope components on the top of the basalt, are roughly 200 feet per mile in a westerly direction and about 50 feet per mile in a northerly direction. However, in the vicinity of wells 3, 8 and

9, the slopes on top and bottom of the basalt seem to vary, at least locally, and the basalt roughly doubles in average thickness from about 54 feet in the wells toward the west to over 100 feet in these three wells, and 86 feet at Well 6 about one mile away toward the northwest.

Latest (most recent) (1977-1978 Pump Testing Results:

Year	Well #		et(Inches) al Draw Down GPM	Gals.Per Min.Yield Per Foot Draw Down	Relative to original
1978	. 1	1 (4'5")) 4.42 100	0 226.24	+
1978	2	2 (12'9"	12.75 100	0 78.43	-
(1977)	3	7 (30')	30.00 73	5 24.50	-
1978	4	9 (70'4")	70.33 80	0 11.37	_
1978	5	4 (20'10")	20.83 110	0 52.80	-
1978	6	3 (20'1") 20.08 117	58.52	+
1978	7	5 (27')	27.00 125	0 46.30	+
1978	8	8 (58')	58.00 80	0 13.79	_
1978	9	6 (26')	26.00 117	0 45.00	+

First,	(Original)	or	01dest	Pump	Testing	Records	on Hand:	

1968	1	1	(6'4")	6.33	1100	173.78
1968	2	2	(13'8")	13.67	1230	89.98
1974	3	5	(18')	18.00	520	28.89
1968	4	7	(47'2")	47.17	900	19.08
1968	5	3	(13'3")	13.25	950	71.70
1968	6	4	(27')	27.00	1350	50.00
1971	7	6	(731)	73.00	1718	23.53
1974	8	8	(68')	68.00	1250	18.38
1976	9	9	(90 ^t)	90.00	1500	16.67

Very likely, the city has its own well ratings and these should be compared with the preceding two tables.

Relative to the pump tests data from wells #1 and #2 and wells #3 and #9, these respective well pairs locations are about (?) 500' apart according to the maps. Data suggests in 1977 and 1978, that #3 pump may have been pumping when #9 well was tested. Obviously, because of close proximity the draw-down cones of depression may overlap each other when these wells are simultaneously in production. Approximate similarities are indicated at wells #1 & #2. Is this a possible explanation or are there other aspects involved?

Is the Nu Way Laundry well still being pumped and if so at what rate and from what depth?

Static Water Level Statistics

Well Numbers	1974-1978 Average	1974-1978 Year Average	(1977-1978)
1 & 2	-7.0 feet	-1.75 ft./yr.	-6.125 ft/Avg.
3, 4 & 6	-17.5 "	-4.38	-6.0 "
5	+17.3 " (?)	+4.33 " (?)	-3.8
7	-11.0 "	-2.75 "	-8.0 "
8	- 8.0 "	-2.00	-6.0
9 (1976-1978)	-31.0 "	-7.75 "	+1.0 "

Question regarding the 17.3 foot rise in static water level at well 5 over the 1974-1978 period should be raised and explained if possible.

Present average annual rate of lowering of the water table appears to be something around 1.5 to a little over two feet per year in the area immediately around Douglas. This is compared to estimates of something from three feet to

over 3.5 feet per year in the irrigated farming areas of McNeal and Elfrida to the north.

Well records over the years are often not perfectly consistent and therefore there may tend to be some questions regarding trying to make completely absolute determinations from them. These apparent inconsistencies are probably partly caused by periodic variations in rainfall and runoff in relation to the particular times the records were taken. Mechanical and chemical changes in the wells, pumps and casing for a variety of reasons can have effect. Also, an occasional measurement, technique or recording error will occur. Otherwise, the records provided by the city suffice quite nicely for the important fundamental and relative considerations made in this report.

Respectfully submitted,



WEH:mt

Discussion Sketches:

- 1. Schematic Plan, 1" = 2000' scale.
- 2. Projected Schematic Section (-N75° E., from near Wells 1 & 2 to near Well 8.)

GEOEX #1295

2 October 1978

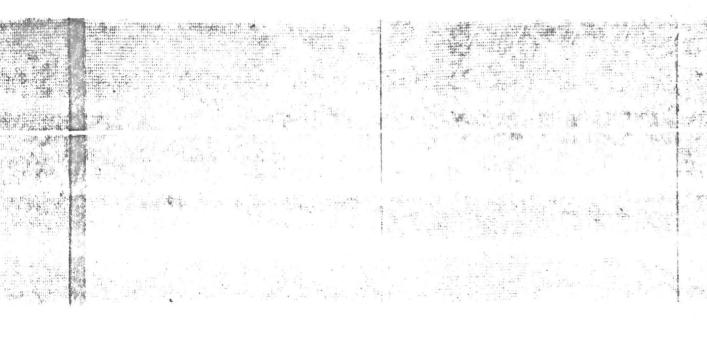
Box 5964

Tucson, AZ 85703

(602) 623-0578

true when considering non-renewable energy resources. In other words, precise determination as to when we are due to run out is not nearly as relevant as what we are doing now to delag comming out or to find new replacements.

An example of such continuing governmental futility, is some of the stated needs for the currently invisited Department of Lieung, such as to more accurately invisited accurately invisited accurately invisited accurately invisited accurately invisited.



NOTE

These corrected pages are to replace page 4 in the two report copies you already have.

Also, please cross out completely the reference to Well 5 on page 2 of the report on lines 6 and 7 so that it reads: - - - "and why well 4 is relatively poorer."

OCT 1 1 1978

October 10, 1978

The City of Douglas Water and Sewer Department P.O. Drawer 1198 Douglas, AZ 85607

Attention: Mr. George D. Nalley, Superintendent

Re: Water System Supply Study

GEOEX #1295

Transmittal Letter

After assimilating these data, I will look forward to meeting again with you Call if the ove gentions, the MEINRICHS, IR. and the Board whenever mutually convenient. ATTACHED CONTENTS Page Summary Letter Report, 10 October 1978 Introduction. General Conclusions & Recommendations Drilling. Geophysics Preliminary Geo-hydrological Analysis, 2 October 1978 Report, Introduction Observation and Speculation . . .

Two-Discussion Schematics, plan and profile - attached.

October 10, 1978 The City of Douglas Water and Sewer Department P.O. Drawer 1198 Douglas, AZ 85607 Attention: Mr. George D. Nalley Superintendent Re: Water System Supply Study Geoex #1295 Summary Letter Report Introduction This letter confirms the conference held in your office on Tuesday 3 October 1978, attended by yourself, members of your department, Joseph Cornejo and Bill Mercer and myself. Receipt of the data, which accompanied your letter of 20 September 1978, and additional material received from your office on 2 October 1978, are acknowledged. The latter includes the drillers logs on holes 9A, 9B, and 9C. General Conclusions and Recommendations Based on all of the above information, on relevant matter procured from the U.S.G.S. in Tucson, and material from GEOEX files, a report titled Preliminary Geo-hydrological Analysis was prepared. This is the report which was referred to during our conference at your office. Two carbon copies of this report were left with you for further reference and three corrected machine copies are being resubmitted herewith. Conclusions arrived at from this initial study are: 1. Generally, wells nearest White Water Draw are best for quantity and perhaps worst for quality, except that quality may also improve to some extent in an up-stream direction, and -12. that some wells producing from above the basalt are better producers than some wells producing from below the basalt, but that probably more directly important, are certain as yet undefined paleo-stringers or channels and permeability factors associated with depositional age and formational character and history in general, rather than the basalt in particular.

Important expeptions to any generalizing are indicated by the relative quantity favorability of wells 5 and 6 and unfavorability of well 4. Careful comparison of the drill logs does not provide any definite explanations. There is some temptation to speculate that factors associated with the exposed or

quantity favorability of wells 5 and 6 and unfavorability of well 4. Careful comparison of the drill logs does not provide any definite explanations. There is some temptation to speculate that factors associated with the exposed or surface trend of the basalt may be responsible, but then, what about relatively fair producing wells 9 and 3 which are closest of any wells to the exposed basalt? Also, capacity deterioriation of Well 4 from 1968 to 1978 suggests that it may need recompletion, clean out and rehabilitation if this has not been done before. Are plugged perforations or screens possible?

Obviously, better quantitative data is necessary before we can begin to answer these tougher questions. One important means to initiate achieving this, in future drilling operations, is to insist on more thoroughly detailed and quantitative drill logs, preferably logged by a competent geologist as drilled and not after drilling or just by the drillers as drilled. In addition, in order to minimize time variation questions and the element of human interpretation from one individual logger to another, calibrated standard "electric" or instrumental well logging is to be highly recommended. This would definitely help to begin sorting out the quandry suggested in the previous paragraph and mentioned later in discussing the sulfide water situation encountered at site 9A.

As a general rule, regardless of how good a well is logged, truly full

evaluation of larger capacity wells, can only be made by adequate pump testing - usually for about a twenty-four hour period. Then, if the well is propert ly perforated, cased, sealed, and packed, maximum life and output efficiency will be achieved. Moreover, all of the quantitative data acquired, including logging, will materially assist and benefit picking the best future well sites and providing for the best future overall program of system planning and development.

For the time being, the initial recommendation to move any immediate or near future new drill site considerations toward a northwesterly direction away from the presently developed area, still stands. This approach should get further away from the basalt and perhaps more into younger and more permeable formations. Holes which avoid the basalt entirely should be cheaper to drill and as long as any particular site doesn't get too close to White Water Draw, present indications are that the quality should be satisfactory and the overall quantity potential generally improved.

Current Program Considerations

After discussing the first report and other pros and cons to be considered with Mr. Nalley and staff, a tentative program for early consideration and implementation was mutually outlined as follows.

Well 8 is presently the well located farthest easterly from the valley center and originally, when first pump tested in 1974, was the second worst producer in terms of gallons per minute yield per foot draw down, i.e.: 18.38 versus 16.67 for well 9 which was originally tested in 1976. Since 1974, well 8 deteriorated to 13.79 gallons per minute yield per foot draw down in 1978. This is still the second worst production factor of the nine wells tested in 1978, and is only slightly better than well 4 which in 1978 tested at 11.37 gallons per minute per foot draw down. Well 8 apparently has a congenital

history of electrical-mechanical difficulties and problems. Because of this and the relatively rapid means and lower cost of pulling, inspecting and, depending on results of that, considering the application of appropriate improvement and/or re-development measures and pump testing, such activity is tentatively set as immediate priority one.

The rationale of this approach is to maximize utilization of present

The rationale of this approach is to maximize utilization of present facilities at least cost, and to better help confirm to everyone's satisfaction whether or not hole 9B is warranted as <u>priority number two</u>. This assumes, of course, that hole 9B is straight and can be satisfactorily completed, perforated and cased if it is not already.

If well 9B is satisfactorily developed, a minimum of new pipe line would be required to establish it into the system. Location of well 9B, although fairly close to wells 3, 8 and 9, is still probably far enough separated to not likely cause any immediate mutual interference and perhaps none for some time to come. If this procedure proves at all successful, then additional assured new capacity will be provided with a minimum of required capital investment.

It is regrettable that Well 9A was abandoned before being thoroughly pump tested. The well log definitely indicates that perched water was encountered at 140-144 feet and that the true or "permanent" water table was encountered at 275 feet - right where it should be. Most likely the reported "sulfide" taste and odor was from the perched water. Although such water also could have come from one or more of the deeper qquifters in the hole, no similar problems have been encountered in any of the producing wells to date. In any case, badwater aquifers can usually be positively identified and located by intertumental logging of the hole and cemented off. Probably in this case they should have

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Hole 9C probably lacked about 40 to 50 feet or so of penetrating the basalt and was abandoned crooked at 260 feet after much reported effort to straighten. Because of that and the close proximity to wells 3 and 9, no further consideration of this site is recommended.

In the preliminary report, a mistake was made on the draw down footages used as shown in the tables on page 4. These have been corrected together with the relevant changes in conclusions in the report copies accompanying this report. The most significant change caused by this correction is an upgrading of wells 5 and 6 from the previous rating. The importance of this is that it tends to enhance indicated possibilities for favorable site development in the S.E. 1/4 of Section 7; the east half of Section 18 and all of Section 17.

Drilling

There was some discussion of the relative merits of rotary versus cable-tool drilling. Technically, rotary should be preferable, but practically, cable-tool results can be as good or even better if the rotary facilities provided are inadequate for properly handling the minimum requirements - such as a straight hole, penetration of hard basalt formation, etc. Theoretically a good rotary drill and driller can perform a fair pump test by using compressed air, while cable-tool bailing tests are generally inadequate for testing

larger capacity wells. Whatever choice is made, the detailed specifications called for by the City and those offered by the drillers should be very carefully examined and weighed and included in any ultimate contract. Pump testing, well preparation, perforating, gravel macking, cementing, casing and pump installation etc., procurement should follow similar procedures.

Geophysics

Surface surveys of electrical resistivity and induced polarization geophysics have been mentioned. It is possible for such surveys to be able to differentiate areas of tighter and or more backish subsurface zones from more porous, permeable and fresher water zones. This is best done usually by correlating definitive test survey results from over good producing well areas with definitive test survey results from poor producing well areas and then making appropriate comparison surveys over new or unknown areas of interest.

This technique does not always work everywhere and unfortunately absolute determination of that can only be established by trial and error. Although the Douglas area appears generally amenable for this type of work, brackish perched water such as possibly encountered at well site 9A can be misleading. Also, culture features such as pipe lines, fences, power lines, etc., can practically prohibit cost-effective geophysical surveys.

Unless the problems become much more acute than presently indicated, geophysical work is not considered worthwhile for the moment.

GEOEX #1295, 10 October, 1978 Box 5964, Tucson, AZ 85703 (602) 623-0578

WEH:mt

WALTER E. HEINGLEFIRE. HEINGLEF



HEINRICHS GEOEXPLORATION COMPANY

P.O. BOX 5964, TUCSON, ARIZONA 85703. 806 WEST GRANT ROAD, PHONE: (602) 623-0578

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HEINRICHS GEOEXPLORATION COMPANY

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been cemented off regardless so as to avoid possible contamination of the other primary aquifers. Possibility of redrilling this hole should be considered and posed to the drilling fraternity for comment. The site is ideally located for easy tieing into the present system and it is not near any other wells. There is no evident reason at this point why this site should not provide at least a fair producer.

Hole 9C probably lacked about 40 to 50 feet or so of penetrating the basalt and was abandoned crooked at 260 feet after much reported effort to straighten. Because of that and the close proximity to wells 3 and 9, no further consideration of this site is recommended.

In the preliminary report, a mistake was made on the draw down footages used as shown in the tables on page 4. These have been corrected together with the relevant changes in conclusions in the report copies accompanying this report. The most significant change caused by this correction is an upgrading of wells 5 and 6 from the previous rating. The importance of this is that it tends to enhance indicated possibilities for favorable site development in the S.E. 1/4 of Section 7; the east half of Section 18 and all of Section 17.

Drilling

There was some discussion of the relative merits of rotary versus cable-tool drilling. Technically, rotary should be preferable, but practically, cable-tool results can be as good or even better if the rotary facilities provided are inadequate for properly handling the minimum requirements - such as a straight hole, penetration of hard basalt formation, etc. Theoretically a good rotary drill and driller can perform a fair pump test by using compressed air, while cable-tool bailing tests are generally inadequate for testing

larger capacity wells. Whatever choice is made, the detailed specifications called for by the City and those offered by the drillers should be very carefully examined and weighed and included in any ultimate contract. Pump testing, well preparation, perforating, gravel packing, cementing, casing and pump installation etc., procurement should follow similar procedures.

Geophysics

Surface surveys of electrical resistivity and induced polarization geophysics have been mentioned. It is possible for such surveys to be able to differentiate areas of tighter and or more brackish subsurface zones from more porous, permeable and fresher water zones. This is best done usually by correlating definitive test survey results from over good producing well areas with definitive test survey results from poor producing well areas and then making appropriate comparison surveys over new or unknown areas of interest.

This technique does not always work everywhere and unfortunately absolute determination of that can only be established by trial and error. Although the Douglas area appears generally amenable for this type of work, brackish perched water such as possibly encountered at well site 9A can be misleading. Also, culture features such as pipe lines, fences, power lines, etc., can practically prohibit cost-effective geophysical surveys.

Unless the problems become much more acute than presently indicated, geophysical work is not considered worthwhile for the moment.

GEOEX #1295, 10 October, 1978 Box 5964, Tucson, AZ 85703 (602) 623-0578

WEH:mt



November 21, 1978 The City of Douglas Water and Sewer Department P.O. Drawer 1198 Douglas, AZ 85607 Attention: Mr. George D. Nalley, Superintendent Re: Water System Supply Study "Phase II" GEOEX #1295 Dear George: We have your letter of 14 November 1978. I understand your decision about not drilling a test hole. Under present circumstances, the cost aspects alone are a little difficult to argue against - especially when such costs stand a good chance of not directly reducing the cost of a subsequent production well. Right now the only factors to go on in choosing one part of New Well Site Two Area over another part are as follows: 1. The farther east you go, the deeper the water table (presently approximately 3800' elevation) will be below the surface (roughly 50 to 60 feet per mile) in the area of interest - thus requiring more well, lift, and casing, etc. 2. The farther east and south you go the better the chances are for thicker basalt. Indications so far suggest a rough increasing thickness factor of about 35 feet per mile, but this could change very suddenly and most probably it would be thicker rather than thinner and probably the farther away from the nearest tie point you get, the better is the chance for a sudden change to occur. 3. Since the local formations appear conformably bedded and dipping or slightly tilted westward then, as you go eastward and deeper to the water table you will be gradually intersecting increasingly older formations. As a generalization, and without specific and contrary information or data to go on, this is apt to mean proportionately decreasing permeability of the aquifer, i.e.: de-

City of Douglas Mr. George Nalley November 21, 1978 Page Two

creasing yield.

- 4. Much of the SE 1/4 of Sec. 8 and most of Sec. 16, the airport section, T. 24 S., R.28E., has basalt at or very near the surface. This may make for more costly and difficult collaring and drilling of a straight hole especially if the churn drilling method is used.
- 5. Existing system considerations as to location of reservoirs, size of pipe line already in and of other producing well sites. At this point, and as a general consideration for now, it is not recommended that producing wells be located closer to each other than roughly 1/2 mile apart unless of course there is some other major overriding consideration involved.
- 6. Detailed demographic factors as to where added supply is apt to be needed most and soonest.

In lieu of complete and detailed consideration of items 5 and 6 above, my personal choices right now are tentatively the corner of 5th and Washington (NW corner of Sec. 17), or 15th and Airport (NE corner of Sec. 17).

Indications from your existing wells suggest a specific capacity of roughly 50 gallons per minute per foot of draw down should be achievable at these three sites if properly drilled and prepared. Regarding the latter, the enclosed article from the March-April 1978 issue of The Johnson Drillers Journal by George H. Lesch may be of interest. The only local small company that seems to have successfully specialized in this technique are Ventures Drilling Company of Tucson - (see their pamphlet enclosed).

Local estimates on churn holes are \$22.00/foot 12 inches diameter, \$32.00 per foot 16 inches diameter and \$40-44 per foot for 20 inches diameter for holes 500 to 600 feet deep with about 100 feet of basalt. One rotary-air estimate for the same specification of hole was \$40 to \$45 per foot for a 15" to 16" diameter hole and with the added statement that 12 3/4" (casing in 16" hole) will nicely handle a 10" bowl pump - rated at 500 to 900 gpm capacity.

If you will give me your pro and con thoughts on these various trade off details as you see them, I would like to assist in working up a proposed well drilling and testing contract and specifications which presumably would ultimately be let out for bid. I gather from previous discussions that there is considerable doubt if much sealing off of the aquifers has occurred in your drilling operations to date. If we could be absolutely categorical about this, then I would feel better about eventually trying to do some quantitative subsurface differential geology and geophysics. Otherwise, right now, I would prefer seeing an equivalent investment put into improved drilling and well development techniques at this stage. Somehow, I cannot help but feel that with all of the reported clay in the drillers logs that the newer and deeper wells besides being in older and tighter formation, did lose some potential yield during the actual drilling process and whether or not they were drilled rotary or churn.

City of Douglas
Mr. George Nalley
November 21, 1978
Page Turee

We finally received a frosted mylar transparent original of the map of Douglas today from the State Department of Transportation. What a struggle it was!

Your and Mrs. Scott's help in straightening out my latent confusion on which part of which holes were drilled churn versus rotary and where the various troubles occurred and what remedies were attempted, etc., will be appreciated. Also, the same for your current drilling cost estimates to compare with those I have acquired here.

I will look forward to any comments you may have or which may be made by any of the board members.

Sincerely,

Heinrichs GEOEXploration Co.

Walter E. Heinrichs, Jr. Geol. Engr., P.E. & G.P.G.S.

WEH:mt

P.S. To other well site choices in addition to the two others already mentioned in Paragraph four of page two of this letter, are near 8th Street and Washington and near the center of section 17, at 8th Street and Jackson. Geologically, from what we know now, both of these sites seem slightly preferable to the site at 15th Street and Airport.

GEOPHYSICAL LOGGING OF WATER DEPARTMENT ROTARY DRILLED EXPLORATION AND PRODUCTION HOLES

Because of the recent controversy and confusion involving geophysical logging of rotary drilled holes, I have undertaken to review the situation. Recommendations were solicited from Ed Davidson of the USGS and Don Madsen. Attached to this review is an appendix containing brief explanations of the various types of logging. The numbers in parentheses contained in the recommendations refer to the numbers used in the appendix.

Ed Davidson recommends:

- A) For slim holes (which will not be converted into production holes) drilled in areas where little or no data is available spontaneous potential (1); induction resistivity (4); porosity log either a borehole compensated formation density log or a borehole compensated neutron log. (These logs are adjusted for the varying width of the hole and are obtained by running a caliper log (9) in conjunction with either a formation density log (6) or a neutron log (8)); sonic log (5).
- B) For production holes he feels that since we have not made use of the logs to blank off undesirable formation, no logs were needed. However, he did concede that when the well was being drilled in an area where only a limited amount of information is known, that a spontaneous potential (1) and some sort of resistivity (2) (4) (10) log could be run.

Don Madsen recommends:

- A) For all slim and pilot holes, regardless of the area in which they are drilled spontaneous potential (1); multiple curve resistivity (2).
- B) For production holes in new areas spontaneous potential (1); multiple curve resistivity (2) or induction resistivity if available; caliper log (9); neutron log (8); sonic log (5).
- C) Production holes in known areas no logs.

Johnson Drilling Company recommends:

Spontaneous potential (1); multiple curve resistivity (2); gamma ray log (7). They use these in combination with a driller's penetration rate log. They believe that the gamma ray log is the best for locating clay beds. This is a Michigan based firm and it is difficult to say if this

would be the most effective group to use here. In the Michigan area the clays probably tend to occur in isolated formations, rather than widely disseminated, as in this area.

Of course, it is to be understood that grain size samples are to be kept and a grain size analysis log is available for use in conjunction with the geophysical logs.

My own recommendations are that for each slim or pilot hole, regardless of location, a spontaneous potential log (1) and multiple curve resistivity (2) log be run. (If available, an induction resistivity log (4) is preferable to a conventional resistivity log (2)). These logs aid in interpretation of the grain size log. Under no circumstances should a point resistivity log (10) be permitted to be substituted for a multiple curve resistivity or induction resistivity log (4). The point resistivity log (4) does not permit the calculations of water quality, porosity, and permeability that are possible from the other types of resistivity logs.

When the slim hole or pilot hole is in an area where little or no hydrologic data is available, consideration should be given to running two additional geophysical logs, a porosity log as defined by Ed Davidson, and a sonic log. (5). Among things to be considered are: the additional cost of such logging; the amount of additional information to be gained and its value to the Water Department. Only a limited amount of geophysical logging has been done in this basin, seeking hydrologic information. Even in other areas, the use of geophysical logging in the search for fresh water is still in the developmental stages. Our ideas of which type of logs to run and the information available from them will change as time goes on.

Walt Stein Sept. 1970

APPENDIX

This section contains a brief summary for each kind of geophysical log which might be run in an exploration or production water well.

The descriptions for the first eight types of logs are taken from Log Interpretation Principles by Schlumberger.

1) The Spontaneous-Potential (or SP) curve is a recording versus depth of the difference between the potential of a movable electrode in the borehole and the fixed potential of a surface electrode.

The SP is useful in holes filled with fresh muds to:

- 1. Detect the permeable beds.
- Locate their boundaries and to permit correlation of such beds.
- 3. Determine values of formation-water resistivity, R_{w}
- Give qualitative indications of bed shaliness (or clay).

The SP is generally recorded on Track 1 (left-hand track) of the log, usually in conjunction with resistivity surveys, but it may also be recorded along with other logs, such as the Sonic Log.

Opposite the shales (clays) the readings of the SP curve are usually fairly constant and tend to follow a straight line on the log, called the shale (clay) base line. Opposite the permeable formations, the SP curve shows excursions from the shale (clay) base line; in thick enough beds they often tend to reach an essentially constant deflection defining a sand line. The deflection may be either to the left (negative) or to the right (positive), depending mostly on the relative salinities of the formation water and of the mud filtrate.

The position of the shale (clay) base line on the log recording has no useful meaning for interpretation purposes. The SP sensitivity scale is chosen and the shale (clay) base-line position is set by the engineer running the log so that the SP curve deflections remain in the SP track.

2) <u>Conventional Resistivity Logs</u>. (ES) (Multiple Curve Resistivity).

During the first quarter century of well logging, the only electrical surveys available were the conventional resistivity logs plus the SP. Thousands of them were run each year in holes drilled all over the world. Since then, new logging methods have been developed to measure values much closer to $R_{\times O}$ or R_{t} , which are the values sought. Nevertheless, the Conventional ES (Electrical Survey, consisting of SP, 16-inch Normal, 64-inch Normal, and 18'8" Lateral) is still being run in many parts of the world.

In conventional resistivity logs, currents are passed through the formation via certain electrodes, and voltages are measured between certain others. These measured voltages provide the resistivity determinations. So that there will be a current path between electrodes and formation, the sonde must be run in holes containing electrically conductive mud or water.

In homogeneous, isotropic formation of infinite extent, the equipotential surfaces surrounding a single current-emitting electrode are spheres. The voltage between an electrode situated on one of these spheres and one at infinity is proportional to the resistivity of the homogeneous formation, and the galvanometer deflection corresponding to such voltage can be scaled in resistivity units.

3) The <u>Laterolog</u> method of resistivity measurement minimizes the influence of the borehole and of surrounding formation by forcing the measuring current to flow radially, as a thin sheet of current, into the formation being logged.

The Laterolog is therefore much superior to conventional resistivity devices (ES log) for resolution of formations of moderate-to-small bed thicknesses. It is also superior for determination of R_t in case of large resistivity contrasts (R_t/R_s or R_s/R_t) and in case of large R_t/R_m values (salt muds/or resistive formations).

4) The Induction Log was developed to measure formation resistivity in boreholes containing oil-base muds. Electrode devices do not work in these non-conductive muds, and attempts to use wall-scratcher electrodes proved unsatisfactory. Experience soon demonstrated that the Induction tools had many advantages over Conventional ES for logging wells drilled with water-base muds.

Induction Logging devices are focused in order to minimize the influence of the borehole and of the surrounding formation. They are also designed for deep investigation and reduction influence of the invaded zone.

Practical Induction sondes include a system of several coaxial transmitter and receiver coils. However, the principle can be understood by considering a sonde with only one transmitter coil and one receiver coil.

High-frequency alternating current of constant intensity is sent through the transmitter coil. The alternating magnetic field thus created induces secondary currents in the formations. These currents flow in circular ground-loop paths coaxial with the transmitter coil. These ground-loop currents, in turn, create magnetic fields which induce signals in the receiver coil. The receiver signals are essentially proportional to the conductivity of the formations. Any signal produced by direct coupling of transmitter and receiver coils is balanced out by the measuring circuits.

The Induction Log operates to advantage when the borehole fluid is an insulator -- even air or gas. But the tool will also work very well when the borehole contains conductive mud, provided that the mud is not too salty, the formations not too resistive, and the borehole diameter is known.

5) The Sonic Log is a recording versus depth of the time ∆t, required for a compressional sound wave to traverse one foot of formation. Known as the interval transit time ∆t is the reciprocal of the velocity of the compressional sound wave. The interval transit time for a given formation depends upon its lithology and porosity. Its dependence upon porosity, when the lithology is known, makes the Sonic very useful as a porosity log. Integrated Sonic transit times are helpful in interpreting seismic records.

Sonic tools in current use are of the BHC (borehole compensated) type. This type sonde substantially eliminates spurious effects at hole-size changes as well as errors due to sonde tilt.

The BHC system uses one transmitter above and one transmitter below two pairs of sonic receivers. When one of the transmitters is pulsed, the sound wave generated enters the formation; the time elapsed between detection of the first arrival at the two corresponding receivers is measured. The speed of sound in the Sonic sonde and in the drilling mud is less than that in the formations. Accordingly, the first arrivals of sound energy at the receivers correspond to sound-travel paths in the formation near the hole wall.

The BHC transmitters are pulsed alternately, and \triangle_{t} values are read on alternate pairs of receivers. The \triangle_{t} values from the two sets of receivers are averaged automatically by a computer at the surface. The computer also integrates the transit-time readings to obtain total travel times.

Sometimes the first arrival, although strong enough to trigger the receiver nearer the transmitter, may be too weak by the time it reaches the far receiver to trigger it. Instead, the far receiver may be triggered by a different, later arrival in the sonic wave train, and the travel time measured on this pulse cycle will then be too large. When this occurs, the Sonic curve shows a very abrupt and large excursion toward higher Δt values; this is known as "cycle skipping". Such skipping occurs only when the signal is strongly attenuated by unconsolidated formations, formation fractures, gas saturation, or aerated muds.

The Formation Density Log, also known as a Gamma Gamma Log, is useful as a porosity-logging tool. Other uses of density measurements include identification of minerals in evaporite deposits, detection of gas, evaluation of shaly (clay) sands and complex lithologies, and determination of oil-shale yield.

A radioactive source, applied to the hole wall in a shielded sidewall skid, emits medium-energy gamma rays into the formations. These gamma rays may be thought of as high-velocity particles which collide with the electrons in the formation. At each collision a gamma ray loses some, but not all, of its energy to the electron, and then continues with diminished energy. This type of interaction is known as Compton Scattering. The Schlumberger source and detector are so designed that the tool response is predominately due to this phenomenon. The scattered gamma rays reaching the detector, at a fixed distance from the source, are counted as an indication of formation density.

The number of Compton-scattering collisions is related directly to the number of electrons in the formation. Consequently, the response of the Density tool is determined essentially by the electron density (number of electrons per cubic centimeter) of the formation. Electron density is related to the true bulk density, ρ_b , in gms/cc, which in turn depends on the density of the rock matrix material, the formation porosity, and the density of the fluids filling the pores.

7) The Gamma Ray Log is a measurement of the natural radioactivity of the formations. The log is therefore useful in detecting and evaluating deposits of radioactive minerals such as potash or uranium ore.

In sedimentary formations the Gamma Ray Log normally reflects the shale (clay) content of the formations. This is because the radioactive elements tend to concentrate in clays and shales. Clean formations usually have a very low level of radioactivity, unless radioactive contaminants such as volcanic ash or granite wash are present, or when the formation waters contain dissolved potassium salts.

The Gamma Ray Log can be recorded in cased wells, which makes it very useful in completion and workover operations.

The Gamma Ray Log can be recorded in cased wells, which makes it very useful in completion and workover operations. It is frequently used as a substitute for the SP in cased holes where the SP is unavailable or in open holes where the SP is unsatisfactory. In both cases it is useful for the location of the non-shaly beds and for correlation.

8) Neutron Logs are used principally for delineation of porous formations and determination of their porosity. They respond primarily to the amount of hydrogen present in the formation. Thus, in clean formations whose pores are filled with water or oil, the Neutron Log reflects the amount of liquid-filled porosity.

A combination of the Neutron Log with one or two other porosity logs yields even more accurate porosity values and lithology identification, including evaluation of shale content.

Neutrons are electrically neutral particles, each having a mass almost identical to the mass of a hydrogen atom.

High-energy (fast) neutrons are continuously emitted from a radioactive source which is mounted in the sonde. These neutrons collide with nuclei of the formation materials in what may be thought of as elastic "billiard-ball" type collisions. With each collision a neutron loses some of its energy.

The amount of energy lost per collision depends on the relative mass of the nucleus with which the neutron collides. The greatest energy loss occurs when the neutron strikes a nucleus of practically equal mass, -- i.e., a hydrogen nucleus. Collisions with heavy nuclei do not slow the neutron down very much. Thus, the slowing-down of neutrons depends largely on the amount of hydrogen in the formation.

Within a few microseconds the neutrons have been slowed down by successive collisions to <u>thermal</u> velocities, corresponding to energies of around .025 electron volts. They then diffuse randomly, without losing any more energy, until they are captured by the nuclei of atoms such as chlorine, hydrogen, silicon, etc.

The capturing nucleus becomes intensely excited and emits a high-energy gamma ray of capture. Depending on the type of Neutron Logging tool, either these capture gamma rays or neutrons themselves are counted by a detector in the sonde.

When the hydrogen concentration of the material surrounding the neutron source is large, most of the neutrons are slowed down and captured within a short distance of the source. On the contrary, if the hydrogen concentration is small, the neutrons travel farther from the source before being captured. Accordingly the counting rate at the detector (with the source-detector spacings commonly used) increases for decreased hydrogen concentration, and vice versa.

Two other types of geophysical logs are also of interest because of their involvment in Water Department contract specifications.

- 9) A <u>Caliper Log</u> records the measurements of the diameter of the hole throughout its depth. Variations in the nature of the formations will cause variations in the diameter of the hole, even though the same size bit is used throughout. Factors which influence hole diameter include: grain size, amount of cementation, caving, and formation thickness.
- 10) Point Resistivity. This was the earliest type of resistivity logging. One electrode is grounded at the surface and the other lowered down the well. The amount of current passing from one electrode to the other is a function of the resistivity of the materials through which the current travels. This method allows only very shallow penetration into the formations, perhaps not beyond the zone disturbed by drilling mud penetration. Also, as each deeper formation is measured, the cumulative effect of this resistivity from the formations above gets larger. No quantitative calculations are possible with the data obtained. These problems led to the development of more sophisticated types of resistivity logging (See 2, 3, 4).

January 15, 1979

The City of Douglas Water & Sewer Department P.O. Drawer #1198 Douglas, AZ 85607

Attention: Mr. George D. Nalley

Superintendent

(GEOEX #1295 cont'd.)
Re: Water Well Drillers,
Drilling Contracts and
Specifications

Dear George:

Reviewing matters more or less covered at the 12-15-78 board meeting and the way I remember they stood at conclusion of that meeting, a decision was made to forego preliminary "slim hole" drilling for purely testing purposes and go directly to a hole for production purposes. It was also decided that some general production well specifications should be drawn up, together with objectives and known facts, for submittal to several prospective contract drillers for the purpose of soliciting proposals from them for drilling the proposed well. I agreed to get back to you on this in January and, hopefully, with additional recommendable contract driller candidates.

So far, I have located at least one more likely candidate presently head-quartered in Willcox, and expect to hear from one or two more this week or next. Some of the qualified rotary drillers are presently involved in multiple hole or deep hole contracts. These of course are of more interest to them than one 600' water well, but I do not think this problem is universal or apt to remain perpetual.

Meanwhile, I have prepared a summary draft letter which generally covers the situation, for your review. This is herewith enclosed and if it is satisfactory, I recommend that copies be dispatched right away to at least:

ROTARY

1. Venture Drilling Company P.O. Box 50325 Tucson, AZ 85703 Telephone No: (602) 623-2211

Recommended relaable by Cities Service, Geo. Kushmaul & others. Only ones encountered so far who specialize in polymer muds for clean holes and maximum production

City of Douglas Attention: Mr. George D. Nalley January 15, 1979 Page Two Three

McBee Drilling Company Room 106, Sands Motel Willcox, AZ 85643

Telephone No: (602) 384-3501

Sounds qualified in talking over phone. Reportedly just completed a 3110 foot irrigation well near Willcox for a Mr. Larry Layton, phone 384-2656. Well not yet successfully tested. Apparently polymer mud was not used. Recommended by Connors Drilling Company who are reliable specialists in diamond core drilling for mining prospecting purposes.

3. Layne Western Company, Inc. 3000 Emory Road El Paso, Texas

Telephone No: (915) 581-5423

1524 (602) 831-5585

1521 So. Suchan Band Rd Tempe, AZ 85281 CABLE TOOL

1. Aithecotton Boring Well Drilling Telephone No: 9401 N. Verch Way (602) 297-4444 Tucson, AZ 85704

(Trustworthy, have known personally for approximately 20 years)

2. Earl H. Williams IDrilling Company Telephone No: P.O. Box 41210 Tucson, AZ 85717

Tucson, AZ (602) 888-325 (David Williams) Hereford, AZ (602) 366-5553 (Earl Williams)

PUMP COMPANIES

1. Gilbert Pump of Tucson, Inc. 2840 Ruthrauff Road Tucson, AZ 85705

Telephone No: (602) 887-1212

(Have done work for the City before)

The Hanson Pump Corporation 2310 W. McDowell Road Phoenix, AZ 85009

Telephone No: Casa Grande (602) 836-7466 Phoenix (602) 258-8111

3. A. L. Cotton Boring Well Drilling Telephone No: (Berkeley Pumps)

(602) 297-4444

(See above under cable tool drillers)

City of Douglas Attention: Mr. George D. Nalley January 15, 1979 Page Two

4. Crow Company, Inc. 4408 E. Speedway Tucson, AZ 85712 Telephone No: (602) 881-0886

(Similar reference as A.L.Boring)

Depending on the drilling character of the basalt, both as first estimated and later as initial drill penetration rates indicate, rotary drillers may start with a small diameter bit to pilot through the basalt and then come back and ream to the final diameter specified. The reason for this is to maximize bottom hole tool pressures when minimum tool weights are available and when the rock is hard. Churn or cable tool drillers may or may not use similar procedures, but they generally have less available means of control to maintain a straight hole under these and most circumstances.

Incidentally, regarding the reported poor tests from the new Agua Prieta well, mentioned at the December 1978 Board Meeting, my records here show that the test pump was set at 220 feet when the test was made. If that is correct, it was set much too shallow - in fact almost right at the static water table level and if that is so, then a good test would not be expected. If this is not the explanation, then the next most likely reason is that natural and/or artificial mud has effectively sealed off the aquifer during drilling, or that geologic conditions in the hole are quite different than encountered in the nearest Douglas city well #5. If Joe Cornejo could get drill logs, or other factual data on the Agua Prieta wells for us, it would definitely be useful. Unless there is some specific reason against it, information trading of this sort can be mutually very worth while.

Sincerely,

Heinrichs GEOEXploration Co.

Walter Cha. Heinrichs. 1.66

Geot. Engr., P.E. & G.P.G.S.

WEH:mt Enclosure

City of Douglas Attention: Mr. George D. Nalley January 15, 1979 Page Four

P.S. With your approval, I would be glad to mail out the letter and screen the responses on your behalf. This would tend to shield you and the City of Douglas if that's desired - at least until final decision is made on specifically who will or who will not be definitely dealt with. Of course, I will need copies of all responses anyway for my evaluation and recommendations. Meanwhile, any comments from your end are most welcome or, if you should want me to come down for more discussion first that's fine too.

Regards.

P.P.S.: You likely have some additional moments of Isilles and funf feofle you may nich to add to may list.

January 15, 1979

The City of Douglas, Cochise County, Arizona

Preliminary Considerations for Proposed Water Well

Drilling Contract and Letter of Solicitation

The City of Douglas, Arizona Water and Sewer Department is contemplating adding an additional well to the present nine well system. Following are the basic considerations and particulars involved.

General Site

Location:

Flat terrain of Douglas Basin, approximately three miles or less from downtown Douglas on city controlled property.

Geology:

Top soil, valley fill or alluvium with a shallow westward and northward dipping basaltic volcanic flow interbed, which is exposed at the surface over much of the eastern portion of the proposed site area. This interbed varies from roughly 50 to 100 feet in thickness where it has been previously penetrated by eight city drill holes located over an adjacent six square mile area. This same area will include or be immediately adjacent to the proposed new well site. Exact location of the new site has not been finally decided but, in any case, it is expected that drilling will have to penetrate roughly 100 feet of basaltic volcanic flow rock. This would be either from the surface down to roughly 100 feet deep in the hole, if the hole is drilled in vicinity of the Douglas airport, Section 16, T24S., R.28E or in Section 8 (northwest of the airport) or in Section 9 (north of the airport).

Specific Site Location and Geology:

If the site is located to the north or west of these said Sections 8, 9 or 16, then the basalt layer will be first encountered at some depth below the surface in the hole and may be thinner than 100 feet thick. The maximum average slope of the top of the basalt, as encountered in eight previously drilled city wells, is about 200 feet per mile from east to west, and about 50 feet per mile from south to north. Thus, the estimated depth to the top of the basalt will be fairly accurately known when the final exact site is chosen. In addition, at that point, there may also be a little more specific data on predicted absolute thickness of the basalt.

Material beneath the basalt, in all of the drilling done to date, is more but older valley fill.

Water Table:

Static water table elevation is at 3790 to 3800 feet above sea level in the present well field. Surface elevations vary from about 3900 feet to 4100 feet. Therefore, depth to the "permanent" water table in the proposed new well site area, will be between 200 feet and 300 feet below the surface. Thus, the static water table may be located above, within, or below the basalt layer (depending on final site location).

Well Specifications

Depending on the final exact site location, the presently maximum programmed well depth is 600 feet. Some kind of pump or air jet capacity and draw down testing is ultimately desired, in addition to setting adequate size and properly perforated casing, gravel packing and final production pump setting and testing. A production capacity of at least 1000 gpm is hopefully achievable but, the range of 500 to 2000 gallons per minute capacity is technically possible based on existing wells.

Based on the above, a straight hole and casing which will comfortably accommodate bowl assemblies of a minimum of 10 inches diameter to a maximum of 14 inches diameter will be required. Therefore, quotes on nominal hole sizes from a maximum of roughly 18 inches final drilled diameter down to as small as 14 inches final drilled diameter and/or whatever increments in between may be chosen, will be considered, but please be specific in indicating alternatives and costs. Depending on pump test results, most likely, a 10" or 12" bowl turbine pump will be installed. However, this could vary if unexpectedly favorable pump test results were obtained.

Proposals:

Your formal or informal quotation or proposal for handling all or any portion of this work is hereby solicited. Please indicate your time estimate, size and type of equipment, materials charges and recommendations such as special muds, etc., - also, such things as your fees, expenses, mobilization, demobilization or travel charges, etc. Date you could start or notice usually required and your estimated incremental times to mobilize, set up, drill, test and/or ream, case, gravel pack, set pump, production test and demobilize, are requested. It is realized that drillers may not wish to quote the production pump setting and testing, but this information is included, because it is of advantage to the city to consolidate as much of the whole project's contracting as is feasible.

The extent to which you will or will not warrant or guarantee hole completion and/or protect the city from lost time and/or equipment losses, in case of trouble or accident in or out of

the hole should be addressed.

Proposals Continued:

If you wish further information you may call or write. Written data from you including certificate of insurance, etc., will of course be required before an award or contract will be let. In any event, some definitive response from you would be appreciated within the next 15 days.

WALTER E. Heinvichs JR., P.E.

Geolici Geophysicist &

RIOLEGATE E. 100

AMERICATE E. 100

ONA. U. S. C. 31.10

Certified Professional Geological Scientist

Proposals Continued:

If you wish further information you may call or write. Written data from you including certificate of insurance, etc., will of course be required before an award or contract will be let. In any event, some definitive response from you would be appreciated within the next 15 days.

A map of the area showing the location of existing wells or holes and the area of the exposed basalt is attached for your information.

Heinrichs GEOEXploration Company

Walter 59 Heinrichs (r) Geola Engra P.E. & C.V.G.S

September 19, 1978 The City of Douglas Water and Sewer Department Post Office Drawer 1198 Douglas, Arizona 85607 Attention: Mr. George D. Nalley Superintendent Re: New Wellsite Hydrological Survey GEOEX #1295 Gentlemen: Thank you for your letter of 18 September 1978, received today. First off, it would be most desireable that I briefly review all readily available factual data that can conveniently be mailed to Tucson right away. A plan map copy or copies showing city controlled lands, by legal subdivision, locations and boundaries, would be most helpful, as would copies of any existing well logs, data and reports from prior work done by or for the city on its own land, as well as any similar data from adjacent or nearby land not under city control but which could still be quite pertinent, such as private land (ranches or Phelps Dodge) and possibly state land. These need not be at all comprehensive or exhaustive and, if bulky or otherwise inconvenient, whatever is immediately and easy to send will suffice, at least initially or for the time being. Whatever you do send will be kept intact and returned after I have finished with them. Should you have any other suggestions, thoughts or questions at any time, please do not hesitate to let me know right away. Meanwhile, I will be looking forward to your response and working for you on this project. Very truly yours, Heinrichs GEOEXplorationCCo. Walter E. Heinrichs, Jr. P.E. & C.P.G.S. WEH: mt

The City of Bouglas

Water & Selver Department Pouglas, Arizona 85607

September 18, 1978



Heinricks Geoexploration Co. Box 5964 Tucson, AZ 85703

Dear Sir:

Referring to your letter of July 21st regarding a hydrological survey relative to locating a site for a new well for the City's water supply.

Will you please arrange to do this job for us. If there is any information you need from us, please let us know, also if we can compile information for you we will do what we can.

Very truly yours,

George D. Nalley

Superintendent

tes

The City of Douglas P.O. Dower 198
Water and Sewer Done

July 21, 1978

New Well-site Agdrological Source

Douglas, AZ 85607

Attention: Mr. George D. Nalley

Superintendent

Gentlemen:

Thank you for your inquiry letter of 17 July 1978 regarding a hydrological survey in connection with considerations relative to drilling a new well for the City of Douglas water supply.

Fees for this type of work are \$25.00 per professional man hour, plus necessary directly incidental job expenses at our invoice cost plus 15%. Transportation is \$17.50 per vehicle day plus \$0.23 per mile per vehicle.

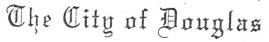
Usual routine is to briefly review and assimilate current and relative facts and data, ideas, problems if any, and possibilities or intended general objectives. This would take one or two office days and if a trip to Douglas is required or desired, about ten hours minimum additional or more, if extenuating factors or special difficulties or circumstances were involved. However, we are aware of none of these and you mention none in your letter and none are therefore anticipated at this stage.

Should for some reason more extensive research or study be indicated, or such work as ground water geophysical methods be needed, then we could proceed either on a day to day basis or preferably, see miendate to msensate fisc proposals, and estimates submitted for your consideration and prior approval.

My estimate at this instant without benefit of all the available facts, is that a \$750.00 - \$1,000 study over a period of 5 to 10 days from start to finish, including one field day at Douglas, would be at least initially adequate and possibly all that would be necessary or recommended. Sometimes we need roughly two or three weeks notice or more before we can schedule new work and proceed with it. Sometimes we can get at it almost immediately. It all depends on our work load at the time.

City of Douglas Mr. George D. Nalley July 21, 1978 Page Two Please do not hesitate to contact us further should you need more details or have further questions. Sincerely, Heinrichs GEOEXploration Co. Walter E. Heinrichs, Jr. P. E. & C.P.G.S. WEH:mt

TELEPHONE (602) 364-8446 (602) 364-8447



Mater & Seiner Department Douglas, Arizona 85607

July 17, 1978

Cable: GEOEX

CALL: GEOEX

101 2 0 1978

BOX 5964 TUCSON, ARIZONA 85703 Phone: (AREA 602) 623-0578

Heinrichs Geoexploration Co. 808 W. Grant Rd. Tucson, Az 85705

Gentlemen:

We are considering drilling a new well for the City of $\operatorname{Douglas}$ water supply.

We would like to have an hydrological survey of the area and would need to know the cost and length of time it takes for this type of work.

Any information you can send us would be helpful. Your prompt attention will be appreciated.

Very truly yours,

George D. Nalley Superintendent

tes

November 2, 1978 The City of Douglas Water and Sewer Department P. O. Drawer 1198 Douglas, AZ 85607 Attn: Mr. George D. Nalley, Superintendent Water System Supply Study "Phase II" Re: GEOEX # 1295 Dear George: This letter confirms the conference held with you, your staff and the City of Douglas Water Board in Douglas on Thursday, October 26, 1978. Interest was expressed in gaining more detailed information regarding the feasibility of developing new production wells further to the east in the same direction as favored by the indicated trends of real estate development. Accordingly, it was requested to consider methods, costs and time required to analize the situation along two profile sections, each one about one and one half miles long, oriented north-south along the eastern and western sides of Sections 16 and 9 respectively and extending from the east-west center line of Section 16 (8th Street) on the south, to the north side of Section 9 on the north. In addition, it was requested that available alternatives and present drilling facilities and costs be preliminarily investigated and recommendations made, especially from the standpoint of test drilling purposes, as opposed to production drilling purposes... Taking the last matter first, more specific investigation is continuing but meanwhile the following generalizations will apply: 1. Churn or cable tool drilling is cheapest, but slowest and will encounter more difficulty with hard-rock formations and especially so in drilling relatively shallower or smaller diameter holes primarily for testing purposes. 2. Churn rigs are commonly only capable of bail testing which is usually inadequate for reliable ultimate production yield estimating for larger yield wells, i.e.; 100 gpm plus.

City of Douglas -2-November 2 1978 3. Ultimate production yield estimates can be most reliably accomplished by major pump testing and associated measurements, however, compressed air jet testing may be a worthwhile compromise under some conditions and this can be done by some available "slim hole" rotary drilling equipment at a basic cost of from \$7.00 to \$10.00 per foot plus extras such as positioning, biodegradable polymer-mud, casing, perforating, etc. 4. Rotary rigs are capable of coolaring in hard rock and maintaining a straight hole with a maximum speed of penetration if properly drilled with a minimum of proper equipment. 5. Rotary drilling usually requires more equipment and people on the crew than does cable-tool drilling. 6. Most expensive method, but one which can provide superior in-hole control and ultimate production yields, is reverse circulation rotary. This is the method currently employed by the City of Tucson. It has also been used by the Salt River Project in the Phoenix area. In both cases this is mainly for production drilling and not for test drilling however. 7. The better rigs and crews seem to be keeping fairly busy and are scheduled six weeks or more ahead. Referring to the two proposed study-profile-sections, one concern is the amount of relief in the older pre-basalt surface. Present indications are that the basalt thickens toward the east gradually and although this could change suddenly, the odds still favor that the basal surface or floor of the basalt will lie above the 3,800 foot water table elevation under the area of Sections 9 and 16. Short of actual test drilling, electrical resistivity geophysics might possibly help map depth to the basalt floor along the two proposed profile sections and give some indirect areal correlation with water productivity potential of the saturated sedimentary aquifer section underlying the basalt. Such work would cost about \$3,000.00. However definitely interpretable and useful results cannot be reliably assured at this point. At a cost similar to the geophysical testing, if done separately or, at a 10% - 20% savings if both were done together, the surrounding exposed sediments which would be expected to underly the basalt under Sections 9 and 16, could be studied and mapped geologically. If the results were clear enough, conceivably useful interpreted projections to depth under the area of Sections 9 add 16 could be made. However, it is also possible that such data would be too obscure or variable or both to either map or project successfully. Success of these indirect non-drilling approaches will depend primarily on whether or not meaningful correlations can be made with existing well data or other factual information as may be acquired later. Of course it is possible that such studies would give some categorical do or don't drill-here answers, but that is much less likely.

City of Douglas -3-November 2, 1978 Summarizing tentatively, one 8" diameter rotary test hole, 600 feet deep, production tested by compressed air, estimate \$7,500.00; geophysics \$3,000.00; geology \$3,000.00; geology and geophysics combined in concurrent survey \$5,000.00; recommended technical supervision, contract negotiation, bidding, etc. \$1,750.00; instrumental logging of drill hole \$2,500.00. Probably not very much could be initiated before around mid-December 1978 even if decided immediately. In the mean time, I should have additional or more refined specific figures for you and meanwhile this letter-report should raise a few questions from your end which I will try to answer right away. Sincerely, Heinrichs GEOEXploration Company Walter E. Heinrichs, Jr. P.E. & C.P.G.S. WEH/jh

The City of Bouglas

Water & Seiner Department Bouglas, Arizona 85607

September 20, 1978



Heinricks Geoexploration Co. Box 5964 Tucson, AZ 85703

Dear Sir:

Referring to your answer to our letter of September 18 regarding the survey for a new well site.

We are enclosing 3 maps of the City, one has the existing wells located on it. A map of the Douglas basin, Logs of our 9 wells and pumping information for the last 5 years. A report from the River Basin-Watershed Planning Staff dated February, 1977.

Also copies of logs of wells in the vicinity. Airport Well, Nu-Way International Laundry Well, Douglas Dees Well, Cochise Junion College Well No. I, El Paso Natural Gas Co. Well, Agua Prieta Mexico well.

Any additional information you need let us know.

Very truly yours,

George D. Nalley Superintendent

tes

2/24/27 Bart ambrose. Dorylos F.O.

River Basin-Watershed Planning Staff Suite 326, Arizona Title Bldg., 111 W. Monroe, Phoenix, Arizona 85003

ENG - Trip Report - Geologic Assistance to Douglas Field Office

February 18, 1977

Ronnie L. Clark RBWP Staff Leader

Tuesday, February 1, 1977, was spent in Douglas providing geological assistance to the Douglas Field Office. The purpose of this trip was to assist the city of Douglas in regard to the development of additional ground water supplies.

Bart Ambrose, District Conservationist, and I met with city officials to discuss the problems confronting the water department in regard to providing an adequate water supply to the residents of Douglas. Personnel in the water department had assembled a considerable amount of data relating to existing water supply and wells which have been drilled. This data greatly facilitated the investigation.

The potential for severe water shortages has been recognized by water department officials. A breakdown of any of the existing wells during periods of high useage would create immediate problems in providing an adequate supply of water to its customers.

A steadily declining ground water level has reduced the yield of some wells, and pumps have been lowered in some wells to accommodate the continued useage of these wells. Fluoride levels in wells numbers 1 and 2 exceed established health department limits for human consumption. Water from these wells has to be diluted with water from other wells before it can be used.

The present water supply is obtained from nine wells. Several other test wells have failed to yield an adequate quantity of water to justify their completion. Wells numbers 5 and 8 pump into a five-million gallon reservoir. The city officials desire to develop an additional source of water to provide water for the storage reservoir. This is the area of 10 7.29.

The existing wells obtain their water supply from the valley-fill sediments and the associated interlayered basalt. The valley-fill sediments consist of gravel, sand, silt and clay of Tertiary and Quaternary Age. The basalt which underlies the city of Douglas is also of Quaternary Age. Although the older sedimentary igneous and metamorphic rocks which underlie and border the Douglas Basin contain some water, they are of no great importance as aquifers since the yield of water from them is generally quite small.

The role of the basalt in relation to ground water movement and yield is not completely known. Available well logs for the city of Douglas indicate that all wells except numbers 1 and 2 penetrated basalt. From the well logs it can be deduced that the basalt flow originated east of Douglas and flowed over an irregular alluvial surface. On this assumption, it is concluded that some of the irregularities on the base of basalt may represent the location of channels cut into the alluvium. This conclusion is substantiated to some extent by the presence of sand and gravel layers which may represent channel deposits under the basalt.

Correlation of individual alluvial strata between the various wells can only be tentative and subject to considerable question due to the lenticularity of alluvial sediments deposited in such a basin as the Douglas Basin. For that reason it is recommended that any additional drilling to develop water supplies should be done by drilling small diameter test holes. These holes should be completed at considerably less cost than large diameter wells required for production. After running pumping tests, a decision can be made as to the advisability of attempting to complete a production well at a given location. In undertaking the drilling of any test well a good log of materials penetrated should be obtained. Collection and preservation of well cutting samples should be required. The running of electric logs on test holes should prove to be of great value in the evaluation of a given test well.

With regard only to interpreted geologic conditions, several areas were selected as meriting further investigation by the drilling of test wells. No consideration was given to the availability of property for test sites or the location of existing water lines to collect and distribute water which might be developed. The locations need not be exact as existing data is not adequate to make very precise determinations.

Tentatively recommended areas for test wells are: (1) near the center of Section 12, Range 27 E, Township 24 S, (2) center of NEW of Section 13, Range 27 E, Township 24 S, (3) near the center of Section 7, Range 28 E, Township 24 S, and (4) near the center of Section 18, Range 28 E, Township 24 S. (See attached map.)

These sites are selected on the basis of geologic conditions as inferred by available well logs. Data obtained from drilling test wells at any specific location should be evaluated prior to initiating drilling at other locations.

R. L. Clark

If test drilling is undertaken it is recommended that the test holes in Sections numbers 12 and 18 be drilled to a depth of about 500 feet. Wells in Sections numbers 7 and (13 should be drilled to about 600 feet. These are tentative depths and should be subject to change based upon information obtained during drilling.

Further water development in the immediate vicinity of Douglas will contribute toward increasing the rate of ground water level decline.

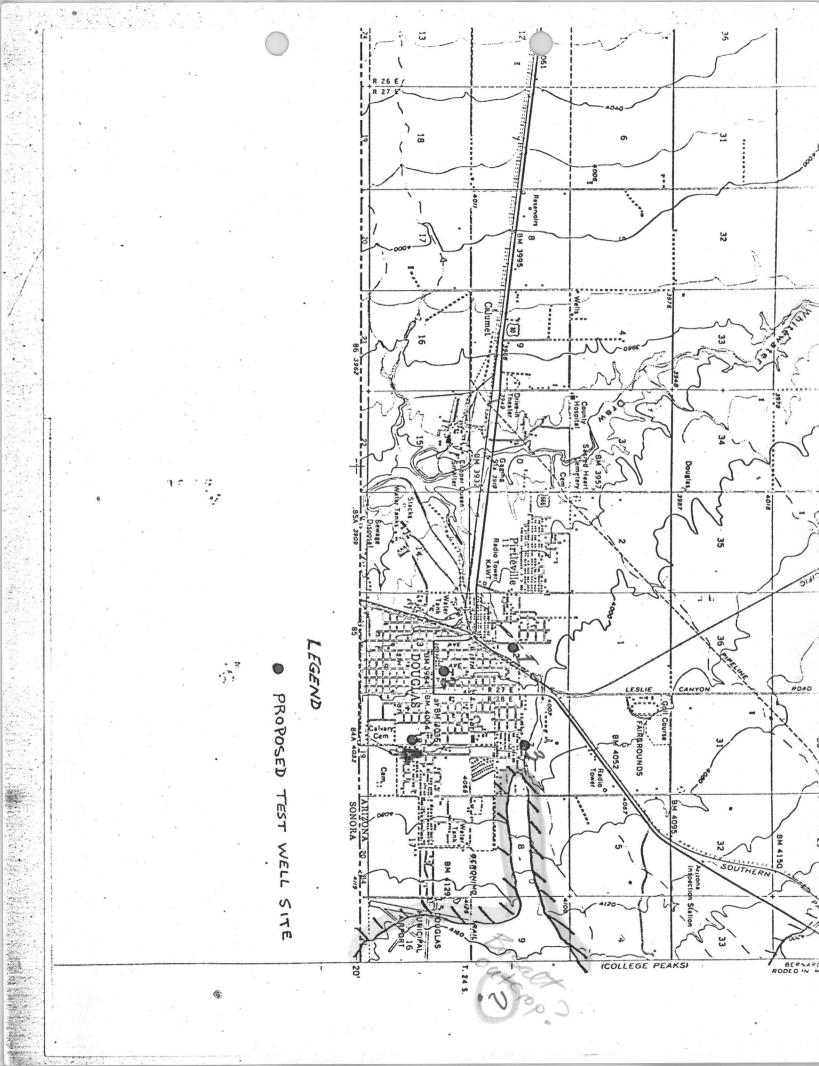
Future water needs may necessitate the development of water supplies from the outlying area. In order to plan for the most efficient development for long range needs, a more comprehensive geohydrological study should be undertaken.

Aubrey C. Sanders, Jr. Geologist

Attachment

cc: (w/att.)
John W. Peterson (1 copy)
Ralph M. Arrington (1 copy)
Joseph L. Knisley, Jr. (1 copy)
Barton E. Ambrose (6 copies)

3



SUPERINTENDENT'S REPORT -- SEPTEMBER 1978

Listed below is the well information, and a comparison of the last five years.

PUMP #1

LOCATION:

Pumping Plant, west of Douglas.

DRILLED: DEPTH:

1942 337'

Cook - 8" Turbine

EQUIPPED: PUMP SETTING:

160'

SUCTION:

10'

TOTAL SETTING:

170'

AIR LINE:

170' 1974 4'-7"

STATIC WATER LEVEL:

DRAW DOWN:

GPM: PUMPING LEVEL ABOVE

TOTAL SETTING:

1975

123'4" + 127'9" -45 129'

_"121'3"

1977

129'3"

140' 1000

1000

1000

130'-8 1000

1331811-4-5 1000

301

PUMP #2

LOCATION:

Pumping Plant, west of Douglas.

DRILLED: DEPTH:

1943 340'

Cook - 8" Turbine

EQUIPPED:

PUMP SETTING:

130'

SUCTION: TOTAL SETTING: 16

146

1974

1975

1976

1977

1978

STATIC WATER LEVEL:

DRAW DOWN:

GPM:

TOTAL SETTING:

118'6"

119'3"

PUMPING LEVEL ABOVE

129-10 4 128 16 11 -11 91 1301-15 140' - 11.2 1200 1000 1000

1000

116'7" + 115'

1321 - 12-9 1000 - 200

16'

PUMP #3

LOCATION:

Overlock Add - 25th Street and Louis Avenue

DRILLED:

1947

DEPTH:

3201

EQUIPPED:

Byron Jackson 10" Turbine was replaced with Gould 8" Turbine

PUMP SETTING:

312' Corrected 9-76 and 20' in 12-76

SUCTION:

10'

TOTAL SETTING:

322'

AIR LINE:

320! Sounding Tube

215'6"

520

233'6"-18

1977

735

1978

STATIC WATER LEVEL:

DRAW DOWN:

PUMPING LEVEL ABOVE

TOTAL SETTING:

730

1974 1975

222 - 34

900

2561

661

SUPERINTENDENT'S REPORT -- SEPTEMBER 1978

PUMP #4 15th Street Park LOCATION: 1956 DRILLED: 461' DEPTH: Johnson 8" Turbine EQUIPPED: 340' PUMP SETTING: 16' 8" 101 SUCTION: 350' TOTAL SETTING: 340' AIR LINE: 1978 1977 1974 1975 1976 " 212 9" 73 7" 286 7 73 7" 224'8" 218 STATIC WATER LEVEL: 2951-704 272'6" 64 6 290 - 72 DRAW DOWN: 800 850 800 850 950 GPM: PUMPING LEVEL ABOVE 501 TOTAL SETTING: PUMP #5 8th Street Park LOCATION: DRILLED: 1960 430' DEPTH: Johnson 8" Turbine. Aug. 76, 40' extension installed. EQUIPPED: 340 PUMP SETTING: 10' SUCTION: +17 4" 350 TOTAL SETTING: 1976 1977 1975 1974 1+ 234' -20' + 219'10" + 216'8" 7 254' -20' + 245'-25' 2 240' 23' 2 220'6" 238' STATIC WATER LEVEL: 241'4"-20/0 2571-19 DRAW DOWN: 1190 1100 1100 - 50 1100 1150 GPM: PUMPING LEVEL ABOVE 110' TOTAL SETTING: PUMP #6 Near Golf Course LOCATION: 1964 DRILLED: 450' DEPTH: Fairbanks Morse - 8" Turbine **EQUIPPED:**

EQUIPPED: Fairbanks Morse - 8" Turbine
PUMP SETTING: 250'
SUCTION: 10'
TOTAL SETTING: 260'

1978 1977 1975 1976 1974 218'-23' -219'10" -228'8" 241' 245'-252 252'8" 214' STATIC WATER LEVEL: 241 - 27 DRAW DOWN: 1175 1175 - 75 1250 1200 1190 GPM:

714"

PUMPING LEVEL ABOVE
TOTAL SETTING:

SUPERINTENDENT'S REPORT -- SEPTEMBER 1978

TOTAL SETTING:

```
PUMP #7
LOCATION:
                      11th Street and Pan American
DRILLED:
                      1967
DEPTH:
                      5541
EQUIPPED:
                      Layne Turbine 10"
PUMP SETTING:
                      230'
    SUCTION:
                      10'
TOTAL SETTING:
                      240'
AIR LINE:
                      240'
                          1974
                                       1975
                                                    1976
                                                                1977
                                                                             1978
STATIC WATER LEVEL:
                          157'
                                    1-158
                                                 1+155'
                                                              - 160°
                                                                           _168' Airline
                          193' -36
                                                   198'-43
                                       188'
                                                                1851-25
                                                                             195' Airline
DRAW DOWN:
                          1150
GPM:
                                       1000
                                                    1250
                                                                1250
                                                                             1250 100
PUMPING LEVEL ABOVE
                                                                             551
TOTAL SETTING:
PUMP #8
LOCATION:
                      27th Street and Washington
DRILLED:
                      1971
                      500
DEPTH:
                      1972-Peerless 8" Turbine
EQUIPPED:
PUMP SETTING:
                      390'
                      10'
    SUCTION:
TOTAL SETTING:
                      400
AIR LINE:
                      407
                          1974
                                       1975
                                                   1976
                                                                1977
                                                                             1978
                                68 0 252'
                                       317' -65'
                                                               + 254
STATIC WATER LEVEL:
                          252
                                                  -256
                                                                            260' Airline
                                                                3201
                          320
                                                    3301
                                                                             318' Airline
DRAW DOWN:
                                                                850
                          1250
                                       830
                                                    900
                                                                             800
GPM:
PUMPING LEVEL ABOVE
TOTAL SETTING:
                                                                             108
PUMP #9
LOCATION:
                      Overlock Addition
DRILLED:
                      1976
                      503'
DEPTH:
EQUIPPED:
                      Goulds 10" Turbine
                      352'
PUMP SETTING:
    SUCTION:
                      10'
                                                    FIRST
                      362'
TOTAL SETTING:
                      340'
AIR LINE:
                                                    TEST
                                                    PUMP
                                                                             1978
                                                    1976
                                                                1977
                                                             256' -25' +255' Airline
281' -25' Airline
STATIC WATER LEVEL:
                                                    224
DRAW DOWN:
                                                    1500
                                                                             1170 -300
GPM:
                                                                1100
PUMPING LEVEL ABOVE
```

86'

81'

CITY OF DOUGLAS WATER & SEWER DEPT. DOUGLAS, AZ 85607

WELL LOG

WELL # 1

STATE I.D. (D 24-27)10-1 PUMP HOUSE - SEC. 10, NW of SE, Township 24, Range 27

DEPTH:

3337 Feet

DRILLED:

September, 1942 by J. L Eicks

CASED:

12" to 277 feet

WATER LEVEL: 57 feet - depth to water 280 12/15/42

LOG

01 to 14' Surface Soil

to 25' 14* Sand, gravel, water

to 75' 25 Clay

75 to 80' Sand, gravel, water

108 to 268' Clay (HD-129') (1978)

2681 to 2771 Brown rock, water

2771 to 337 Blue lime rock, water

DECEMBER, 1958 Repairs - J. R. Sharp - columns were lengthen 20 feet: total depth 334

12" I.D. - 0" to 248' - 8" I.D. 248' to 334' Casing:

Perforated full length 10" bell welded on top

of 8" liner.

WATER LEVEL: 55 feet

MAY, 1966 - WATER LEVEL - STATIC 90*1"

96'8" DRAWN DOWN

February, 1967 881911 Static Water Level

Gals. pumpaper Min.

Drawn Down

92'10"

MAY, 1968 STATIC WATER LEVEL

100' 106'4"

DRAWN DOWN

1100 g.p.m.

MAY, 1970 STATIC WATER LEVEL

105'

DRAW DOWN

1081

GALS. pumped per m.

990

STATE I.D. (D 24-27)10-2

WELL # 2

PUMP HOUSE - SEC. 10, NW of SE,

TOWNSHIP 24, RANGE 27

DEPTH:

340 Feet

DRILLED:

February, 1943 by J. L. Eicks

CASED:

12" Standard T & G 13 Casing

WATER LEVEL: 55 Feet

LOG

100 000

0' To 20' Soil

20' to 30' Sand, Gravel, water

30' to 83' Clay

83' to 95' Gravel (water)

95' to 120' Clay

= (Had 119')(1978)

120' to 130' Gravel (water)

130' to 200' Clay & Gravel

200' to 260' Clay, Sand, Gravel

260' to 335' Black gravel (Water)

335' to 340 Red Clay Gravel

DECEMBER, 1958 Repairs - J. R. Sharp - columns were

lengthen 20 feet. total depth 336'

CASING: 12" I.D. 0' to 250' - 8" I.D. 250' to

336', perforated full length 10" bell welded on top of 8" liner

MAY, 1966 - WATER LEVEL - STATIC 90'8"

FEBRUARY, 1967 - Static Water Level 80'8"

Draw Down 91'5"

MAY, 1968 STATIC WATER LEVEL 89'

DRAWN DOWN 102'8"

Gallons Pump per min. 1230 g.p.m.

MAY, 1970 STATIC WATER LEVEL 107'
DRAW DOWN 112'

Gals. pumped 1100 g.p.m.

```
WELL # 3 - OVERLOCK
               May, 1947 - Joe L. Eicks
DRILLED:
               North half of Block 28 - Overlock Addition
LOCATION:
              SE &, E &, of SW &, Section & Township
               24S, Range 28E. G & S.R. B & M Cochise
                             Lots- 1. thros + 4 ghul)
               County
               320
DEPTH:
                             Lots - 13 thro 24 -
        LOG OF WELL
        2
               Top Soil
     - 15
  2
               Black Soil & Boulders
 15
     - 35
               Gravel & Clay
     -118
 35
               Red Clay
118
     -122
               Gravel
122 - 160
               Sandy Soil
                                             Static Water
                                       165'
160 - 165
               Sand gravel, water
                                             Level
165 - 170
               Clay & Gravel
170 - 175
               Clay & Gravel
175 - 190
               Clay & Boulders
190 - 200
               Sand, (water)
200 - 211
               Porous Malapai
211 - 227
               Hard Black malapai
227 - 231
               Porous black malapai
231 - 242
               Hard black malapai
242 - 248
               Porous malapai
248 - 258
               Hard black malapai
258 - 267
               Porous Red Malapai with water crystals
267 - 280
               Extra hard black malapai
280 - 283
               Porous Malapai
283 - 291
               Extra hard black malapai
291 - 306
               Hard black malapai with crevices full of
              mud
306 - 317
              Porous malapai with water crystals
317 - 320
              Tight hard mixture of clay & gravel like concrete
```

STATE I.D. (D 24=28)18-3

STATE I.D. (D 24-28) 7-4 WELL # 4 - 15TH STREET PARK

	WELL	# 4 - 15TH STREET PARK	
	DRILLED:	January, 1956 - Valley Tr	actor & Equip. Co
	LOCATION:	Section 7, Twp. 24S, Range	5.
		SW ઢ, SE ջ, SW ջ, 15th St	. Park
	DEPTH:	461"	
	LOC	G OF WELL	
	0 - 3	Top Soil	
	3 - 53	Red Clay	
	53 - 90	Brown Clay	
	90 - 145	Red Clay	
	145 - 170	Red and Brown Shale	THE RESIDENCE OF THE PROPERTY.
	170 - 190	White Sand /	
	190 - 240	Clay & Gravel	- 227' (1970)
	240 - 270	Sand & Gravel	
	270 - 280	Boulders	The second secon
	280 - 335	Malapai Rock 55 5/2/	
	335 - 461	Sand & Gravel	477
	150	Static Water Level	
-			
	MAY, 1966 - W	ATER LEVEL - DRAWNDOWN 18	8'6"
	FEBRUARY, 196	7 - Static Water Level -	- 177'4"
		1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	225' returns
		within 25 minutes.	1 11
	MAY, 1968		2'6" 47-2
		A Property of the second secon	9'8"
		Gals. pumped per min.	900 g.p.m.
M	MAY, 1970	STATIC WATER LEVEL	227'
		DRAW DOWN	245'
		Gals. Pumped	800 g.p.m.
A	PRIL, 1971 S	STATIC WATER LEVEL	

800

DRAW DOWN G.P.M.

225 (978)

STATE I.D. (24-28)18-5

	WELL # 5 8TH STREET PARK
DRILLED:	August, 1960 - J. R. Sharp
Location:	Twp. 24S, Range 28E, Section 18
	SE t, NE t, SWt, 8th St. Park near 5th St
DEPTH:	430'
	LOG OF WELL
0 - 60	Red Clay with Caliche streaks
60 - 80	Rocky clay
80 - 105	Red Clay
105 - 110	Rocky Clay
110 - 135	Red Clay
135 - 240	Rocky Clay (197 420) (971)
175	Rocky Clay first water (Not very strong) (971)
240 - 264	
264 - 270	Red Malapai 58
270 - 298	Black Malapai
298 - 430	Gray Conglomerate
169	Static Water Level
MAY, 1966	WATER LEVEL - DRAWNDOWN 202'11"
FEBRUARY,	1967 Static Water Level 184'5"
	Draw down 198'
MAY, 1968	STATIC WATER LEVEL 190'9" 13-3
	DRAWN DOWN 204'
	Gals. pumped per min 950 g.p.m.
MAY, 1970	Static water Level 195'6"

DRAW DOWN

DR^{AW} DOWN

G.P.M.

APRIL, 1971

Gals. Pumped

STATIC WATER LEVEL

210'

197'

210'

1000

950 g/p/m.

(220,5'-1978)

STATE I.D. (D-23,28) 6-6 WELL # 6 - NEAR THE GOLF COURSE

DRILLED:	May,	1964	_	J.	R.	Sharp
----------	------	------	---	----	----	-------

LOCATION: Township 24S, Range 28E, Section 6

SE 4, SW 4; of NW 4

DEPTH:

450

LOG OF WELL

0 - 160	Red	Clay	&	Boulders

160 - 190 Sandy Clay & Gravel

190 - 290 Conglomerate 203 H20 (2335-1978)

290 - 294 Sandy clay & Gravel

294 - 345 Grey Malapai

345 - 380 Red Malapai

380 - 450 Conglomerate

180.5 Static Water Level

221' Drawidown at 1400 G.P.M.

May, 1966 Water Level - Static 186'2"

Drawmdown 189'

FEBRUARY, 1967 -- Static Water Level - 187'6"

Drawm Down 219'8"

MAY, 1968 STATIC WATER LEVEL 195'

DRAWN DOWN 222'

Gals. pumped per min. 1350 g.p.m.

MAY, 1970 STATIC WATER LEVEL 200'

DRAW DOWN 228'

Gals. Pumped 1300 g.p.m.

APRIL, 1971 STATIC WATER LEVEL 203'
DRAW DOWN 233'

G.P.M. 1300

= (1-24-37) 1E-1

WELL	#	7	_	340 -	11TH	STREET	ייHיי	Ave.
------	---	---	---	-------	------	--------	-------	------

DRILLED: MAY	, 1967	-	EARL	LOHN	DRILLING	CONTRACTOR ·
--------------	--------	---	------	------	----------	--------------

Twp. 24 South: Range 27 East, Section 13

NW 4; SE 4; NW 4.

5541 DEPTH:

LOG OF WELL

0	-	5	Surface	Soil	

5 - 25Light Yellow Clay

25 - 85Brown Clay

85 -160 Brown Shale & Clay

H20 142 Gravel (not much) Water 160 -165

165 -215 Brown Some Gravel

215 -385 Brown Clay & Gravel

385 -400 Black Clay Gumbo

400 -410 Black Hard Malapai

Black Extra Hard Malapai 410 -422

422 -450 Black Hard Malapai

450 -470 Red Clay & Gravel

470 -512 Brown Clay & Gravel

512 -542 Gravel

542 -554 Hard Sand & Gravel

Static Water Level 132'

Drawn Down pumping 1718 G.P.M. recovery 2051

to 134'6" in three minutes.

STATIC WATER LEVEL APRIL, 1971

DRAW DOWN

185 '

G.P.M.

1250

-25.339 m/ Adpointment

STATE I.D. D(24-28)8 Bbb / WELL #8 EAST OF WASHINGTON AVENUE \$ 7,616.00 JUNE, 1971 - EARL LOHN, JR. Twp. 24 South; Range 28East; Section 8 NW 4, NW4, NW4 500 Feet LOG OF WELL SOIL CALICHE SAND CLAY AND GRAVEL SANDY CLAY SAND AND GRAVEL SAND-GRAVEL WITH CLAY BLACK MALAPAI ** BROWN (Very Hard) BLACK 11 Brown (Very Hard) **Black** Brown Brown Burnt Clay Black Malapai (Very Hard) Brown decomposed malapai Brown Conglomerate Sand and Gravel Brown Conglomerate Hard Conglomerate Brown Conglomerate

Conglomerate with hard streaks

Hard Conglomerate

Brown Sticky Clay

Static Water Level

DRILLED:

LOCATION:

3

DEPTH:

6 - 12

12 - 27

27 - 55

55 - 95

95 - 122

122 - 163

163 - 167

167 - 173

173 - 190

190 - 196

196 ~ 208

208 - 210

210 - 224 224 - 259

259 - 295

295 - 302

302 - 319

319 - 321

321 - 480

480 - 488 488 - 498

498 - 500

230 1

0 -

- (260'-1978) (H20)

STATE I.D. D(24-28)7 acc WELL # 9 - OVERLOCK

DRILLED:

JULY, 1976 - EARL LOHN, JR.

LOCATION:

SOUTH half of Block 28 - Overlock Addition NW 1/4, SW 1/4, of NE 1/4 Section 7, Township 24S, Range 28E, G & S. R. B & M Cochise County.

On Lots 8 and 9

LOG OF WELL

DEPTH:

5031

2 Top Soil

Brown Clay with Boulders

13 Sandy Clay

13 20 Sand

20 42 **Gravel**

42 55 Brown Sticky Clay

55 83 Brown Sandy Clay

83 - 189 Brown Gravel with Clay

189 - 192 Gravel & Boulders

192 - 195 Brown Malapai

195 -- 210 Very Hard Black Malapai

210 - 234 Brown Malapai (1st water @

234 - 260 Black Hard Malapai

260 - 272Brown Malapai

272 - 283Very Hard Black Malapai

283 - 300 Brown Malapai

301 - 306 Extra Hard Black Malapai

306 - 308 Black & Red Malapai

308 - 315 Gravel In Clay

315 - 405 Cemented Conglomerate

405 - 410 Clay & Conglomerate

410 - 503 Brown Cemented Conglomerate

116' (255'-1978)

The City of Bouglas

Water Pepartment . Pouglas, Arizona 85607

31-12 th St. Located on map? March 25, 1966 Un-Way International Laundry - Well Drilled March 23, 1966 By Willcox Pump & Equipment Co. Top Fill, Oily 12 Yellow Clay, Gumbo HBESSEL 9 60 Grey Shell Rock 63 Red Clay, Gumbo 83 90 Sand & Gravel Clay, Gumbo 168 Sand & Gravel, water 185 Yellow Clay 202 Clay W/streaks of sand & gravel & small boulders 248 251 Hard red Clay Perforations 120' to 250'

Basalt: 280' deep 1/2 mile east (City#4)
.: 400' " 1/3 " west ("#7)

Static Water Level 105'
Pumping Water level 151'

Gast of agua Freeta - apprage Trash auc. Sept. 1970

SECRETARI DE RECURSOS H DRAULICOS GERENCIA GENERAL EN EL ESTADO DE SONORA (ZONA NORTE) AGUA POTABLE Y ALCANTARILLADO. AGUA PRIETA, SONORA.

CLASIFICACION LITOLOGICA DE CAMPO DEL POZO No. 3 PARA AGUA POTABLE DE AGUA PRIETA, SONORA.

No water level indicated

PROFUN (pies) De a	DIDAD (Mts.) De a	CLASIFICACION
00.00 - 20.	0 0.0 - 6.00	Arenas, poca gravilla y arcilla (x)
20 - 30	6.00 - 9.15	Arenas, poca gravilla, poca grava y arcilla.
30 - 60	9.15 - 18.30	Arenas, gravilla y arcilla.
60 - 70	18.30 - 21.35	Arenas, gravilla, poca grava y poca arcilla.
70 - 80	21.35 - 24.40	Arenas, gravilla, poca grava y muy poca arcilla.
80 - 100	24.40 - 30.50	Arenas, gravilla y arcilla.
100 - 150	30.50 - 45.70	Arenas, gravilla y grava.
150 - 160	45.70 - 48.80	Grava, arenas y gravilla.
160 - 170	48.80 - 51.80	Gravilla, grava y arena.
170 - 180	51.80 - 54.90	Grava, gravilla y arenas.
180 - 200	54.90 - 61.00	Arenas, gravilla y grava.
200 - 210	61.00 - 64.00	Arenas, gravilla y poca grava.
210 - 220	64.00 - 67.00	Arenas y poca gravilla.
220 - 230	67.00 - 70.10	Arenas, gravilla y poca grava.
230 - 270	70.10 - 82.30	Arenas, gravilla y grava.
270 - 290	82.30 - 88.40	Arena, poca gravilla y poca grava.
290 - 330	88.40 - 100.60	Arenas, gravilla y grava.
330 - 400	100.60 - 121.92	Grava, gravilla, arenas y arcilla.
400 - 460	121.92 - 140.21	Arenas, gravilla, grava y poca aroilla.
		Drate 1 Lane mynes

⁽x) "AREMAS" Comprende: Arena de grano fino, medio y grueso, distribuidas - uniformemente.

ARENA - SAND GRAVILLA - GRAVEL (FINE) AKCILLA - CLAY GRAVA - GRAVEL (COARSE)

UP B

Douglas Dees County Hospital Road Locate on map

Well Log:

O ft. to 3 ft. Too soil

3 ft. to 47 ft. Clay

47 ft. to 51 ft. sand (Virst water)

51 ft. to 71 ft. Sandy clay

71 ft. to 75 ft. Sand and gravel (water bearing)

75 ft. to 180 ft. Clay with large gravel embedded

130 ft. to 183 ft. Sand and gravel (Water bearing)

183 ft. to 223 ft. clay with amall embedded gravel (black, green, red)

223 ft. to 229 ft. Sand and small gravel (water bearing)

229 ft. to 248 ft. Clay

248 ft. to 266 ft. Sand and gravel (water bearing)

266 ft. to 295 ft. Clay with large gravel embedded

295 ft. to 306 ft. Sand and gravel

Total depth of hole-- 306 ft.

Type of casing--steel 8"

Method of sealing at reduction points -- welded

Perforated from 200 ft. to 291 ft.

Size of cuts--1/3" by 12" No. of cuts per ft. 4

Method of construction -- drilled

Date started -- April 8, 1975

Date completed -- April 16, 1975

Drilled by Bobby McDonald

Box 30E

McNeal, Arizona

No Basalt -!

J. R. Sharp

GENERAL CONTRACTOR

BOX 01175 - PHONE HE 2-4597

BISBEE, ARIZONA

COCHISE COLLEGE AUG 3'63

JULY 1963

L C G

COCHISE JUNIOR COLLEGE WELL NO. I Locate on map

DEITH	FORMATTON
0 - 5 feet	Top soil
5 - 12 "	
12 - 18 "	Gravel
	Clay
18 - 26 "	Gravel .
26 - 173 "	Clay
173 - 177 "	Sand and gravelFIRST WATER AT 173 feet
173 - 203 "	Clay
203 - 215 "	Gravel with some clay
215 - 350 "	Tan clay. Thin layers of gravel
350 - 400 "	Alternate thin bedded shales, conglomerate and clay
	no basalt?
0 - 354 "	16" O.D. pipe
163 "	STATIC LEVEL
354 - 400 "	Open hole
172 - 344 "	
	Perforated Mills Knife 3" x 33" 6 each foot
31414 - 3140 m	Perforated Mills Knife 2" x 32" 6 each foot lerforated Torch 3/8" x 6" each foot
	lerforated Torch 3/8" x 6" each foot. One hour bailer test at or
3 ¹ 1 ¹ 4 - 3 ¹ 49 11 215 11	One hour bailer test at 25 gpm at drilling depth of 215 feet. Static level 173 feet. Water
344 - 349 11	One hour bailer test at 25 gpm at drilling depth

The City of Douglas Water Department DOUGLAS, ARIZONA

Des. 1941.

LOG OF AIR PORT WELL

9 and 1/2 miles north of Douglas Locale exact on map

	Feet	from	top	Feet to	
		0'	Top Soil Caliche	1,1/2*	
		41	Clay and small rock		
11-		147	mater, sand & gravel	150	
1100		150	Joint Clay	162 /-20	
		-1621	Water, Sand & gravel	164	
		164	Clay & large gravel	2921	
		292*	Boulders	2971	
		2971	Concrete formation	2991	
		2991	Water, Gravel	328*	
		328*	Clay	3321	
	* 1	332	Water, Gravel	337* . // /	01
			- 4	No basa	ilt.

3" of 10" Casing with 88' of perforation

DRILLED BY JOB EICKS & ONEY REYNOLDS

LICHNSED DRILLERS.

EL PASO NATURAL GAS COMPANY

WATER WELL DATA REPORT

Water Well No. Howard	No. 5 STATION
Water Well Drilled By	$C_1 = 0$
Smilling Completion Date 1946 .	(N.Z. Louglas 7-8 m1.
The state of the s	(N.E. Douglas 7-8 mi. (Sec 9, T. 235, R.28E.)
	(Se-917728 1796)
HORE LATA	(acc 1) 1, 250, K. 282,)
Consider the second	
TAL WELL DEPTH - FERT	400
THE THE TENT (TOD ON POTENTS) trans	10-1 / 5-00 / 50
CHATTC LEVEL (TOP TO BOTTOM) - FEST	110000000000000000000000000000000000000
AMEZING LEVEL (TOP TO BOTTOM) - FEET	
LEAV DOWN - FEET	
FIRST SATISTICS	210 MK
PUMPING CAPACITY G.P.M.	
DATE ABOVE TEST TAKEN	
TEST SUPERVISED BY	
End Dama	
MAKE	
MODEL	
TYPE	Hi-Lift
SATED CAPACITY G.P.M	A * * 2 6 8 6 6 4 8 6 C 6 2 8
SERIAL NUMBER OF PUMP	80739
ROW SIZE	1'
COLUMN SIZE	4 ⁿ
NO. OF BOWLS	1
FOR MOTOR DATA	
MAKE	General Electric
HODEL	
TRATED HP	
AMES	
veers	220/440, 3 phase, 60 cycle
ARIAL NUMBER OF MOTOR	Njj6959333
	· · · · · · · · · · · · · · · · · · ·
CASING DATA (TOP TO BOTTOM)	Type K
Charles and Charles and Charles (TOT TO DOLLOW)	Code F
FT. CASING SIZE 8 5/8	Frame 6503
FT. CASING SIZE	RPM 1755
FT. CASING SIZE	
	Pumping Column
FT. CASING SIZE	21 X 10 210
	Bowles 6
FT. SCREEN INSTALLED	Total 216
FT. PEFFORATION	
SIMBER OF PERFORATIONS PER FOOT	
APPROXIMATE LENGTH EACH PERFORATIONIN.	
TITE TAGING SEAL	
23123	
HOTE: ATTACH DRIVLERS LOG & COPY OF	
WATER ANALYSIS WHERE AVAILABLE	D. C. Kelly
	PREPARED BY
	Dec. 15, 1954
	DATE (2)
	MAD

EL PASO HATURAL GAS COMPANY COMPRESSOR DEPARTMENT

NO. 5 STATION
WELL NO. HOWARD WELL DATE

LOG

TO	FORMATION	NO. FE
35' 50'	Gravel and Caliche Sand	
701	Lime	
110 130	Gravel and Clay	
180	Gravel and Clay	
190 200	Water Sand	
250 360	Hard Gravel	
370 400	Clay and Gravel Fine Gravel and Water Gravel	

El Paso Natural Gas Company COMPRESSOR DEPARTMENT LABORATORY

WELL WATER ANALYSIS

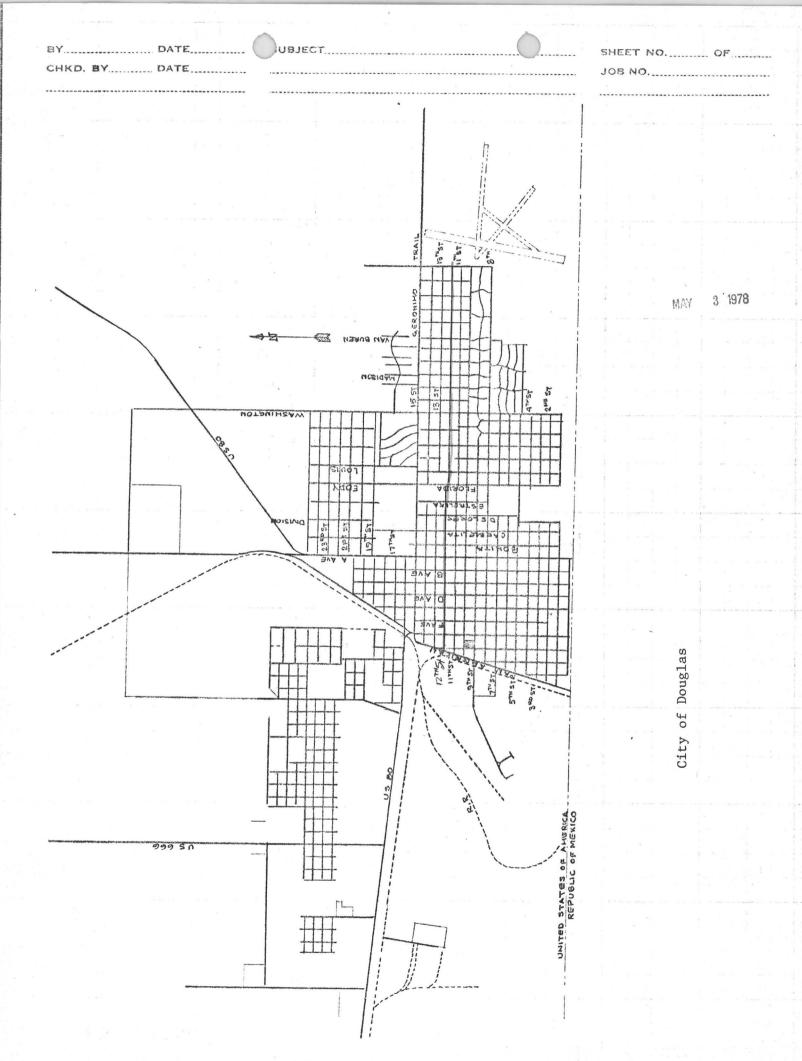
Sample Description - Station No. 5 Howard Well

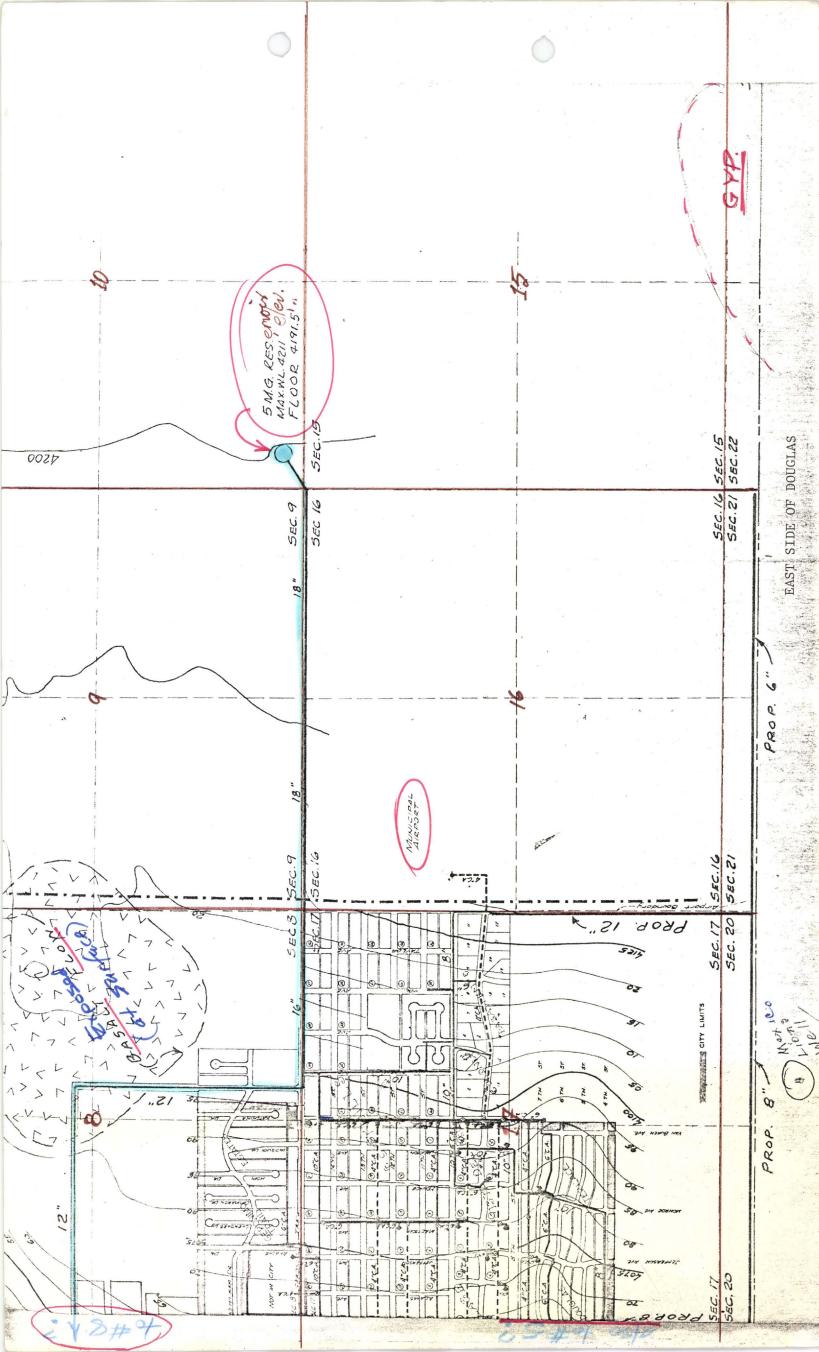
Sample Appearance - and Odor

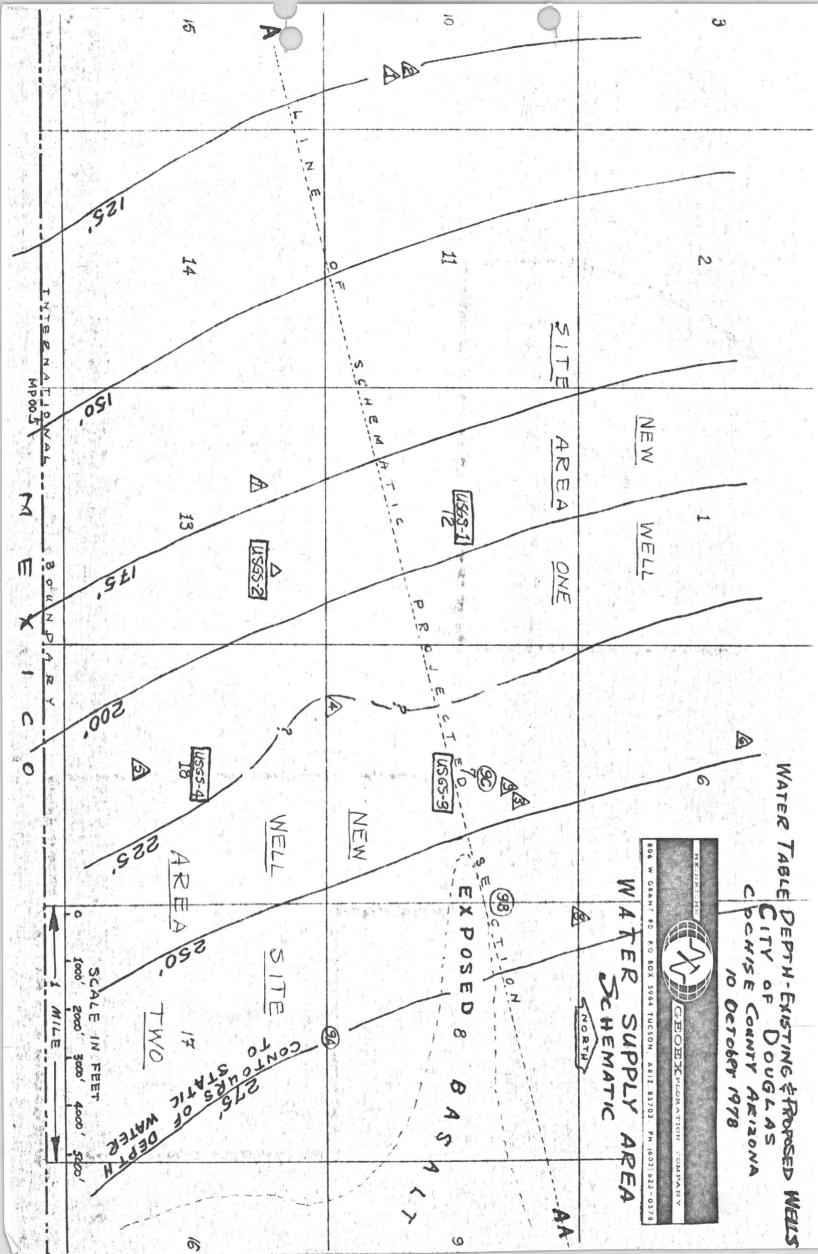
2/3/50	2/6/50	2/13/50		
7.5	7.8	7.8		
148	166	146 -		
104	118	111.		
44	48	35		
0	0	0		
245	250	245-		
32	32	32		
82	90	92		
26	27	27		
0.2	0.2	0.2		
468	468	502		
	7.5 148 104 44 0 245 32 82 26 0.2	7.5 7.8 148 166 104 118 44 48 0 0 245 250 32 32 82 90 26 27 0.2 0.2	7.5 7.8 7.8 148 166 146 104 118 111 44 48 35 0 0 0 245 250 245 32 32 32 82 90 92 26 27 27 0.2 0.2 0.2	7.5 7.8 7.8 148 166 146 104 118 111 44 48 35 0 0 0 245 250 245 32 32 32 82 90 92 26 27 27 0.2 0.2 0.2

ALL RESULTS EXPRESSED AS PARTS PER MILLION

FRMARKS:









Red 10 31

2840 RUTHRAUFF ROAD TUCSON, ARIZONA 85705

May 19, 1977

City of Douglas Water Department Douglas, AZ 85607 Attn: George Nalley

Dear George:

Enclosed is a copy of the results on the test we conducted on Well No. 9, May 18, 1977, under normal conditions. If you can recall and I doubt without any problem the normal conditions did not last too long.

I have made a list of items that need your immediate attention or repair:

- Reset the timer on the starter so that when energized the motor will run in start for no more than three seconds.
- Contact A.P.S. and have them correct the high voltage problem. All three legs read out at 495 Volts when the pump is in operation.
- 3. Install voltage meter in panel (to be installed by Gilbert Pump).
- 4. Realign pump with discharge piping and provide means to secure discharge head to the pump foundation. (This work to be performed by Gilbert Pump after receiving authorization from the City of Douglas)
- 5. Examine all discharge piping for possible cracks or flange leakage due to the stress on the line during the malfunction of the system.

If you have questions or if you have anything further to add to the above please don't hesitate to call me.

Sincerely,

GILBERT PUMP OF TUCSON, INC.

Al Ciasca Sales Engineer

cc: R.L. Sears enc1.

GILBERT PUMP & EQUIPMENT C

PUMP TEST

Tested By: 1. CIPSOR
Date: 5-18 /77 2-76
OWNER City OF DOUGLAS Well #: 9 Power Meter#: 433310
LOCATION:
ASSEMBLED INFORMATION
FLOW METER: SPECIALISTS PIPE I.D.: GPM: 1200
METER KH: 4.8 C.T. MULT.: 80 10 REV. IN 83.5 SEC.
PUMPING LEVEL: 304' FT. STATIC LEVEL Z35' FT.
EXT. HD. OF Z14.8 FT. DRAW DOWN 69' FT.
*TOTAL PUMP HD. 518.8' FT. GAL/FT. D.D. 17.4 GAL.
FRICT. LOSS OF 49 FT. AMPS 225 218 218
TOTAL BOWL HEAD 523.7 FT. VOLTS 495 495 495
PUMP SETTING 352' FT. COL. SIZES 10"XZ'\Z'X\"/\" BOWLS: 9-5TAGE 12 SHMC COL. SIZES
BOWLS: 9-STACE 12 SHMC COL. SIZES
PUMP RPM 1760 MOTOR HP 200
CALCULATIONS
KH 4.8 x MULT. 80 x 36 ÷ SEC. 83.5 10 REVS = KWI 165.
KWI 165.5 x 1.34 = IHP 221.8 x .92 = BHP 204
GPM 1200 ÷ 226 = 5.31 ACRE FT./24 HRS.
GPM 1200 x TPH* 518.8 ÷ 3960 ÷ IHP 221.8 = 0.A. EFF.
OVERALL EFF. (O.A. EFF.) 70.8 ÷ .92 = PUMP EFF. 7
POWER COST
KWI x RATE/HR x 24 = COST/24 HR
24 HR. COST ÷ AC/FT/24 HR = COST/ACRE FT
REMARKS:

GP 07-2M-2-75

Fund threet might 8:30 AM Later + Sewer Dept. 7/8/16 City of Douglas Started testing @ 8:30 Al state Cevel 224 Generalized GIM Time RPM 8:30 A.M. 8:95 -256 1400. 261 700 9:00 266 815 9:15 270 890 9:45 279 1100 10:00 284 1150 10:15 10:30 45. 10:45 11:15 11:30 11:45 12:85 P.M. Recovery from 314-272-1min 10see) fes 372- 224-5~ 1:30 PM 1:45 2'00 2:15 2:30 3:00 3:15 3:30 3/2 319' 4:00 __35__ - AUS 1976 RECEIVED.

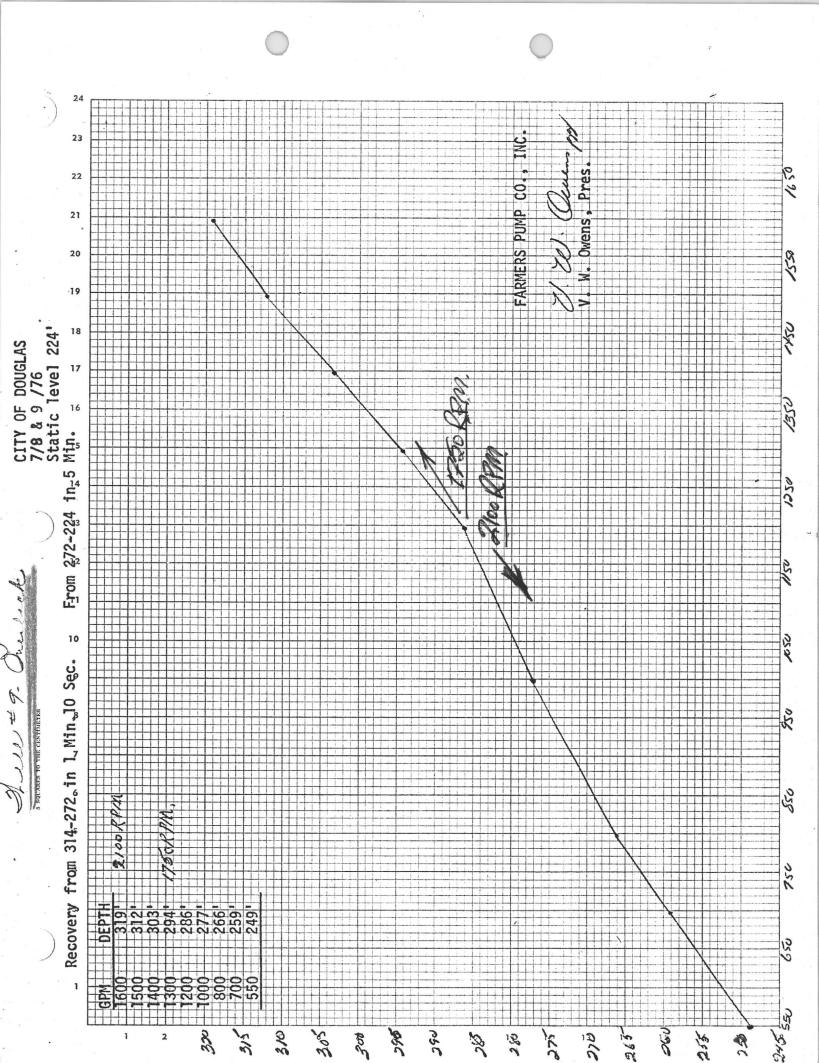


Table 4.2 - Chemical examination of water, City of Douglas Water Department wells, reports of analyses by the Arizona State Health Department Laboratory, March 15, 1972. Values shown are in parts per million.

)									0					
27th St. & Washington	340	204	54	37	50.	178	0.57	œ	78		Colorless		Mary agreement			
340 11th St.	335	10	7 1	103	o. ∞ ;	182 25	9.0	7 %	34 <.01 1930)	∨ ∨ ∨ ∪ 10 m		< .05	8.8	ZN 0.08	
Near Golf Course	278	128	13	47	, 9	12	9.0	2 5	232300) W M		> .05	8.6 .05	ZN 0.08	
8th St. Park	390	55	17	37	0 0 1	20	8.0	2 4	<pre></pre> < .01 1650	\ \ \	v v v		< .05	7.8	ZN .05	
15th Street Park Well #4	333	29	12	54	-	19	9.0	28 4	.011950	· · · · · · · · · · · · · · · · · · ·	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \		> 05	8.4	ZN . 05	
Overlock Well	290	35	12	42 × .05	0	19	1.0	37	< 0.1 2200		V V		> 05	< .05	ZN . 05	
Main Sta. North Well #2	635	4	7 7	194	132	194	1.1	88	0.01	< >	V V		< .05	< .05	ZN . 05	
Main St. South Well #1	650	4	106	< .05	132	198	7.0	06	0.01	S	V V		< .05	< .05	ZN 0.05	
	Total Solids Total Hardness	Calcium	Magnesium Sodium		Alkalinity Phenolphthalein Methyl Purple	Chlorides Fluorides	Nitrates	Sulfates	Chromium Specific Resistance (Ohms)	Turbidity	Color	Special Tests	Manganese PH	Copper	Chemist's Remarks	

				-		•
4,0	COCATION	MATTIC LEVEL	DRAW DOWN	CI.P.M.	DATE & YEAR	TAKEN BY E
1	TZEW 08 YAWIH	121'5"	150	1000.	30 AUG. 77	
11	HIWAY 80 WEST	129'3"	133'8"	1000	13 SEPT 78	FIERROS LEO
2	HIWAY 80 WEST	115'	130'	1000	30 AUG. 77	
24	HIWAY 89 WEST	119'3"	132 '	1000	13 SEPT 78	FIERROS LEON
3 2	OLD OVERIOUS	226'	256'	735	30 AVG 77	
33	ord enserbook	232'8"	/		13 SEPT 78	FIERROS LEON
1	15 T. PARK	2181	290'	800	30 AUG 77	
1 4	15 ¹¹ ST. PARK	224'8"	295'	800	13 SEPT 78	FIERROS LEON
5	8th ST. PARK	216'8"	240′	1100	30 AVG. 77	
,5	8 th ST. PARK	220'6"	241'4"	1100	13 SEPT 78	FIERROS LEON
5	GOLT COURSE	223'8"	252'8"	1175	35 AUG 77	
66	it or Consess	232'9"	252'9"	1175	13 SEPT 78	FIERROS LEON
1	11 th 5T.	160	185'	1250	30 AUG 77	
it	11 th 5T.	168' AIRLIKE	195 ARUKE	1250	(3 SEPT 78	FIERROZ LEON
	271 & WASH.	254'	320'	850	30 AG 77	
8	27th 4WASH.	260' AIRLINE	318 AIRLIUE	800	13 SEPT 78	FIERROS LEON
a	HEW POMP	25('	291'	1100	30 Avg 77	
9	NEW POMP	255 ARLINE	281' ARLINE	1170	13 SEPT 78	FIERROS LEON
	}		A. L.			

CITY OF DOUGLAS WATER AND SEWER DEPARTMENT WATER REVENUE ANALYSIS FOR THE FISCAL YEAR ENDED JUNE 30, 1977

ford 10/3/18

		Meters in Service	Average Revenue Per M Gallons Sold	Monthly Meter Charges	Gallons Sold	Gallons Pumped	Gallonage Not Metered
1976	- July August September October November	4210 4214 4217 4221 4223	\$.4457 .4375 .4424 .5176 .6021	\$ 40,460 37,598 35,054 29,084 26,384	90,786,300 85,943,800 79,235,900 56,187,400 43,818,000	117,515,200 121,178,600 102,950,200 80,577,500 63,097,400	26,728,900 35,234,800 23,714,300 24,390,100 19,279,400
1977	December	4215 4233 4235 4240 4236 4252 4247	.6116 .6210 .6453 .5170 .5406 .6538	27,455 26,315 26,632 30,121 30,490 43,608 55,833	44,887,500 42,378,600 41,271,900 58,260,000 56,399,200 66,699,800 85,157,200	63,192,800 51,331,000 55,911,100 73,479,500 83,012,800 89,921,600 125,017,000	18,305,300 8,952,400 14,639,200 15,219,500 26,613,600 23,221,800 39,859,800
F.Y.E	. 6/30/77	4247	\$.5446	\$409,034	751,025,600	1,027,184,700~	276,159,100
F.Y.E	. 6/30/76 6/30/75 5/31/74 5/31/73 5/31/72	4203 4139 4038 3970 3871	.4786 .4834 .4901 .5241 .4274	393,943 377,415 407,645 362,213 326,766	780,766,900		208,966,100 227,216,300 336,543,600 208,262,200 205,234,200

NOTE: Above figures do not include water used by and charged to the City of Douglas as follows:

- V -	C 120 177	¢01 770	160 455 600
F.Y.E.	6/30/77	\$81,778	168,455,600
F.Y.E.	6/30/76	71,637	170,275,900
F.Y.E.	6/30/75	48,710	73,217,700
F.Y.E.	5/31/74	57,308	152,057,800
F.Y.E.	5/31/73	45,525	104,227,600
F.Y.E.	5/31/72	36,302	100,225,400



CITY	OF DOUGLAS WATER & SEWER DEPT. 425 10th S DRAWER 1198 DOUGLAS, ARIZONA 85607 PHONE 364-8446	MEM-O-GRAM URGENT – REPLY IMMEDIATELY NO REPLY REQUIRED
	Heinricks Geoexploration Co.	DATE: Sept.29, 1978
	Box 5964	SUBJECT:
o	Tucson, AZ 85703	
MESSAGE	Mr. Heinricks:	
	Enclosed are copies we secured from Phelps Dodge Co	orp.
	The copies attached to this note must be returned to The other copies you may keep.	us.
	The other copies you may keep.	
	George D. No	alley D. D. Marley D.
REPLY	ta fer above seed: 10/2/20 Ded accordingly, Many Hans.	and will be
9	RECIPIENT - RETAIN WHITE COPY - RETURN PINK COPY - TO MAIL IN WINDOW E	NVELOPE - USE FOLD MARKS

ALOHA FORM 00-873-1

WELL ABANDONED AT 15TH ST. & VAN BUREN

MARCH, 1975

State ID D924-28)17bas 31325

Driller: Earl Lohn, Jr.

Location: 15th St. & Van Buren - NE 1/4, NE 1/4, NW 1/4,

Section 17, Twp. 24 South, Rge 28E

			LOG	
	0	7	2	Top Soil
	2	-	6	Caliche
	6	-	13	Sandy Clay - Brown
	13		18	Sand
	18	-	47	Clay - Brown
	47	-	55	Gravel
_	55		63	Sandy Clay & Gravel - Borwn
1-	63	don't	83	Gravel & Boulders
1	83	-	99	Malapai - Black
1-	99	-	105	Hard Malapai - Black
	105		109	Malapai - Black
	109		115	Hard Malapai - Black
1	115		117	Malapai - Black
1 ,	117 120		120 122	Very Hard Malapai - Black
171	122	_	125	Very Very Hard Malapai - Black Extremely Hard Malapai - Black
11	125		126	Very Hard Malapai - Black
1.	126		130	
11	130		145	Malapai - Grey Malapai - Brown - Water 140 - 144
V	145		174	Malapai - Brown
	174		210	Conglomerate - Brown
1	210	_	230	Conglomerate - Brown (Possible Decomposed
1	-	Company of the last		Malapai) 7
	230	-	245	Clay with Gravel - Brownish
	245	_	258	Sandy Clay - Brown
	258	-	265	Sandy Clay with Gravel - Brown
	265	-	275	Gravel & Clay
	275		285	Decomposed Rock - Water at 275'
	285	_	300	Decomposed Rock with some Clay
	300		334	Decomposed rock with some clay - Brown
	334		344	Decomposed rock - Brown
	344	-		Hard Decomposed rock - Brown
	351	-	372	Hard Cemented Conglomerate - Brown
	372	-	378	Clay with Boulders - Brown
	378	-	384	Boulder & Clay - Brown
	384	_	394	Hard Cemented C onglomerate - Brown
	394	-	397	Very Hard cemented conglomerate - Brown
	397 443	_	443 445	Hard cemented conglomerate - Brown
				Very Hard cemented conglomerate - Brown
				nally accepted for public usage.
	Not	tes	ted.	oasing sarvaged.

Sylom, bailing Sulfide

Sed: 10/2/28

Well Abandoned at 25th St. and Washington November, 1975

State ID D(24-28)7 ada 33145

DRILLER: Earl Lohn, Jr.

335

338

349

300

335

338

25th St. & Washington NE 1/4, SE 1/4, NE 1/4, Location:

Twp. 24 South, Rge. 28 East Section 7

Block 24 - Lots 13 and 14 Overlock Addition

				STOCK 24 HOUS IS AND IT
			I	.0G
	0		2	Top Soil Sind Son as
	0	_	7	Caliche & Boulders
	7	_	22	Carrene a Bodicers
	22	_	30	Sandy Clay - Brown
	30	-	80	Sandy Clay with some gravel - brown
	80	_	100	Gravel with some clay - brown
	100	_	103	Gravel Gravel
pione accom decision	103		115	Malapai - Black
	:115	-	120	Hard Malapai - Black
	120		125	Malapai - Black
	125	_	127	Very Hard Malapai - Black
	127		131	
	131	_	138	Malapai - Black //0 74 40 /50
	138	-	146	Hard Malapai - Black Malapai - Black Malapai - Dark Brown Malapai - Dark Brown
	146	_	160	Malapai - Brown
	160	-	175	Malapai - Black
	175	_	178	Very Hard Malapai - Black
	178	_	196	Malapai - Brown
	196	_	200	Very Hard Malapai - Black
	200	_	205	Malapai - Black
	205	_		Malapai - Grey
	220	-	235	Very Hard Malapai - Black
	235	-	247	Malapai - Brown
	247	_	249	Extremely hard malapai with some
				quartz - First water - small amount
	249	_	252	Hard malapai - Black
mental a	252	_	256	Gravel (water) - Brown
	256	_	259	Gravel - Brown
	259	_	271	Clay with gravel - Brown
	271	_		Conglomerate - Brown
	283	-		Hard Conglomerate - Greenish-gray
	291			Conglomerate - Brown
	294	_	300	Hard Conglomerate - Grey
				1 C 1 t C /

Very Hard Conglomerate - Gray

Very Hard Conglomerate - Gray

Conglomerate - Gray

over

Hoke 18" fo 339'

CITY OF DOUGLAS WATER DEPT. 425 - 10th ST. DOUGLAS, AZ. 85607

file: 9B contd. LOG 349 361 Hard Conglomerate - Gray 361 404 Very Hard Rock - Gray 404 407 Very Hard Rock - Gray 407 Broken Hard Rock - Gray 416 502 Very Hard Rock - Gray 502 504 Broken Rock - Brownish 504 564 Broken rock with some soft areas - brown 564 584 Broken rock with hard streak at 570 24" Hole to 150' & from top of 6 sett (103')
18" " to 339' & from top of 6 sett (103')
6 5/8" casing installed - 346'10" 6" pilot hole from 349 to 584

WELL ABANDONED IN BLOCK 28 - OVERLOCK MARCH, 1976

Driller: Earl Lohn, Jr. - Bernard Drilling Co.

Location: Block 28 -

			LOG	
	0	-	3	Top Soil
	3	_	10	Sandy Clay - Brown
	10	_	40	Gravel & Boulders - Brown
	40	_	48	Sandy Clay - Brown
	48	_	57	Gravel
	57	_	77	Sand - Gravel in Clay - Brown
	77	-	95	Sandy Clay
	95	-	103	Sand
	103	_	188	Sand, Gravel, Small Boulders in Clay-Brown
	188	_	195	Sand - Gravel
	195	-	208	Sand - Gravel with boulders (Malapai)
A	208	-	222	Malapai - Black - First Water 222 FT.
(-)	222	-	224	Hard Malapai - Black
12	224	-	233	Malapai - Black
V.	233	_	260	Malapai
-				Note concern De Constitution

Hole Crooked

Pump te steat

Chum.

QUESTIONS:

- 1. Relative to the pump tests data from wells #1 & #2 and wells #3 and #9, these respective well pairs locations are about (?) 500' apart according to the maps. Data suggests in 1977 and 1978, that #3 pump may have been pumping when #9 well was tested. Obviously, because of close proximity the drawdown cones of depression may overlap each other when these wells are simultaneously in production. Approximate similarities are indicated at wells #1 & 2. Is this a possible explanation or are there other aspects involved?
- 2. Is the Nu Way Laundry well still being pumped and if so at what rate and from what depth?
- 3. Well 5 1974-1978 Static level +17.3 feet why?
- 4. Exact location of abandoned well (C, and are there any other dry or abandoned locations and data?
- 5. Regarding abandoned wells 9A, 9B, 9C, were none of these pump tested and, except for 9C which couldn't be straightened and was apparently too crooked, why not?
- 6. S.E. Douglas area mentioned as a preferable location if geohydrologically feasible - why is this area preferable?

PRELIMINARY GEO-HYDROLOGICAL ANALYSIS

For

THE CITY OF DOUGLAS, ARIZONA

Sept. - Oct. 1978

by

Heinrichs GEOEXploration Company
P.O. Box 5964 Tucson, AZ 85703

Preliminary Geo-hydrological Analysis Introduction Pursuant to correspondence initiated by Mr. George D. Nalley, Superintendent of the Douglas Water and Sewer Department on 17 July 1978, GEDEX, on 18 September 1978, was engaged to conduct a brief preliminary hydrological survey for the City. Objective of the survey was to assist in the siting of a proposed new water well under consideration for the purpose of augmenting. in the most efficient manner feasible, the present nine-well-system. Data and technical records requested by GEOEX on 19 September, 1978, were received on 22 September 1978. These, together with material already on hand and other pertinent locally acquired information, were studied with results as summarized in this preliminary report. Conclusions and Recommendations: 1. United States Geological Survey (U.S.G.S.) Trip Report of 18 February 1977, copy of which was supplied by the City, tentatively recommended an area (1), proposed to be located near the middle of Section 12, T.24S., R.27E. This is concurred to be a satisfactory primary new well site. 2. Alternatively, anywhere roughly near or within a pie-shaped wedge area bordered approximately on the south by 22nd Street and Merrit Avenue in Pirtleville, and on the east by A Avenue and Leslie Canyon Road, is considered equally satisfactory except for perhaps distribution and rights of way considerations which are not necessarily part of this immediate study. 3. U.S.G.S. proposed areas numbers (3) and (4), are not recommended from the most preferable geologic point of view as suggested by present data on hand. - 1 -

Based on the city records provided, and although there is some exception and variation indicated, there seems to be a definite general quantity decrease of well yields in an easterly direction away from White Water Draw. Therefore, on that basis alone, it appears generally desirable at this point to keep closer to White Water Draw. Possible exceptions to this generalization include a question of why well 6 is a relatively better producer and why wells 4 and 3 Ware relatively pooter. Concflevably, the basalt and the pre-basalt topography and related shallow formations, may be at least partially responsible. If it becomes necessary and important to further pursue these particular types of discrepancies, surface geophysical work, such as electrical resistivity, may warrant testing and could be helpful if tests' results proved favorable. 4. If rights of way and distribution factors outweigh maximum added quantity considerations, then U.S.G.S. area (2) should also be satisfactory and, in some respects may almost equal their area (1). Indicated possible exceptions to this are the proposed site's proximity to poorer well site 4 and its indicated more adverse subsurface conditions, possible interference with well 7, and probably interference with Nu Way Laundry's well if it is still producing and, if so, if that should be a necessary factor to consider.

- 5. Existence of "several other test wells" is mentioned in the U.S.G.S. Trip
 Report of February 1977. Although these are reportedly negative in terms of
 quantity results, the data still should be integrated with the results of this
 study because such information could change the conclusions and recommendations
 herein considerably.
- 6. Long range implications of an anticipated continued increasing rate of ground water level decline and consumptive uses should be kept in mind, both as

to quantity and quality. Ideally, a replenishable surface water system, combined with conservation measures and balanced ground water recharge and withdrawal armangements should be the ultimate objective of all responsible municipalities. Although beyond the scope of this particular study, problems and possibilities along such lines should be investigated and implemented as rapidly as ways and means will allow. Observations and Speculation: 1. At present the less static water table measurement is at an elevation of about 3790 feet near White Water Draw at well #1. This is at about 130 feet below surface. Three and one quarter miles eastward, at Well 8, present static water elevation measures just above the 3800 foot elevation. This represents a rise in static water table elevation of roughly 3 feet per mile from White Water Draw toward the east as far as Well site #8. 2. Top of basalt at Well 4 is 40 feet deeper than at well 5, with a total thickness of 55 feet at well 4 and 58 feet at well 5. This is indicative of a possible small northerly dipping component to the basalt layer and perhaps a somewhat southeasterly source, conceivably, even fairly close by. However, as indicated in U.S.G.S. Trip Report of 18 February 1977, specific significance of the basalt relative to local ground water supplies, if any, is relatively undetermined. 3. Basalt thickness varies from 50 feet at Well 7 to 117 feet at Well 3. It outcrops about 2000' south of Well 8 and roughly 2000 feet southeast of wells 3 and 9. Average locally measured down-slope components on the top of the basalt, are roughly 200 feet per mile in a westerly direction and about 50 feet

per mile in a northerly direction. However, in the vicinity of wells 3. 8 and

9, the slopes on top and bottom of the basalt seem to vary, at least locally, and the basalt roughly doubles in average thickness from about 54 feet in the wells toward the west to over 100 feet in these three wells, and 86 feet at Well 6 about one mile away toward the northwest.

Latest (most recent) (1977-1978 Pump Testing Results:

Year	Well #	(Relative Rank		t(Inches) 1 Draw Down	GPM	Gals.Per Min.Yield Per Foot Draw Down	Relative to original
1978	1	1	(4'5")	4.42	1000	226.24	+
1978	2	2	(12'9"	12.75	1000	78.43	
(1977)	3	7	(30')	30.00	735	24.50	•
1978	4	9	(70'4")	70.33	800	11.37	•
1978	5	4 (:	20'10")	20.83	1100	52.80	•
1978	6	3	(20'1")	20.08	1175	58.52	•
1978	7	5	(27')	27.00	1250	46.30	+
1978	8	8	(58')	58.00	800	13.79	
1978	9	6	(261)	26.00	1170	45.00	+

First,	(Original) or	Oldest	Pump	Testing	Records	on Hand:

1968	1	1	(6'4")	6.33	1100	173.78
1968	2	2	(13'8")	13.67	1230	89.98
1974	3	5	(18')	18.00	520	28.89
1968	4	7	(47'2")	47.17	900	19.08
1968	5	3	(13'3")	13.25	950	. 71.70
1968	6	4	(27')	27.00	1350	50.00
1971	7	6	(73')	73.00	1718	23.53
1974	8	8	(68')	68.00	1250	18.38
1976	9	9	(90')	90.00	1500	16.67

9, the slopes on top and bottom of the basalt seem to vary, at least locally, and the basalt roughly doubles in average thickness from about 54 feet in the wells toward the west to over 100 feet in these three wells, and 86 feet at Well 6 about one mile away toward the northwest.

Latest (most recent) (1977-1978) Pump Testing Results:

	1		Feet Draw Down	GPM	Gals Yield Per Foot Draw Down
1978		11	(4-5")33.67-4.42	1000	7.48 226.24
1978	2	23	(12-9"1)2. 12.75	1000	7.58 78.43
(1977)	3	77	30 256. 30,00	735	2.87 24.50
1978	4	98	(70-4'295. 70.33	800	2.71 //. 37
1978	5	45	(20-10-241.3 20.83	1100	4.56 52.80
1978	6	34	(20-1" 252.75 20.08	1175	4.65 58.52
1978	7	53	(27')195. 27.00	1250	6.41 46.30
1978	8	8.9	(58) 318. 58.00	800	2.52 13.79 -
1978	9	60	(26')281. 26.00	1170	4.16 45.00
	First,	(Original)	or Oldest Pump Testing	Records	on Hand:
1968	1	13	(6'-4"+06.3 6.33	1100	10.35 173.78
1968	2	22	(13-8-102.67 13,67	1230	11.98 89.98
1974	3	58	(18')233.5 18,00	520	2.23 28.89
1968	4	72	(47-2"229.67-47.17	900	-3.92 19.08
1968	1 5	36	13-3"-204. 13.25	950	4.66 71.70
1964	6	44	27' 221. 27.00	1300	-6.33 - 50.00
1971	7	62	73' 134.5 73.00	1718	12.77 23.53
1974	8	88	68' 320. 68.00	1250	-3.90-18.38
1976	9	9.5	90' 314. 90.00	1500	4.78 /6.67

Very likely, the city has its own well ratings and these should be compared with the preceding two tables.

Relative to the pump tests data from wells #1 and #2 and wells #3 and #9, these respective well pairs locations are about (?) 500' apart according to the maps. Data suggests in 1977 and 1978, that #3 pump may have been pumping when #9 well was tested. Obviously, because of close proximity the draw-down cones of depression may overlap each other when these wells are simultaneously in production. Approximate similarities are indicated at wells #1 & #2. Is this a possible explanation or are there other aspects involved?

Is the Nu Way Laundry well still being pumped and if so at what rate and from what depth?

Static Water Level Statistics

Well Numbers	1974-1978 Average	1974-1978 Year Average	(1977-1978)	
1 & 2	-7.0 feet	-1.75 ft./yr.	-6.125 ft/Avg.	
3, 4 & 6	-17.5 "	-4.38 ^a	-6.0 "	
5	+17.3 " (?)	+4.33 " (?)	-3.8 "	
7	-11.0 "	-2.75 "	-8.0 "	
8	- 8.0 "	-2.00 "	-6.0 "	
9 (1976-1978)	-31.0 "	-7.75 "	+1.0 "	

Question regarding the 17.3 foot rise in static water level at well 5 over the 1974-1978 period should be raised and explained if possible.

Present average annual rate of lowering of the water table appears to be something around 1.5 to a little over two feet per year in the area immediately around Douglas. This is compared to estimates of something from three feet to

over 3.5 feet per year in the irrigated farming areas of McNeal and Elfrida to the north.

Well records over the years are often not perfectly consistent and therefore there may tend to be some questions regarding trying to make completely absolute determinations from them. These apparent inconsistencies are probably partly caused by periodic variations in rainfall and runoff in relation to the particular times the records were taken. Mechanical and chemical changes in the wells, pumps and casing for a variety of reasons can have effect. Also, an occasional measurement, technique or recording error will occur. Otherwise, the records provided by the city suffice quite nicesy for the important fundamental and relative considerations made in this report.

Respectfully submitted,

WALTER E. Weinrichs, Jr. HENNECHS IE. P.G.S.

WEH:mt

Discussion Sketches:

- 1. Schematic Plan, 1" = 2000' scale.
- 2. Projected Schematic Section (-N75° E., from near Wells 1 & 2 to near Well 8.)

GEOEX #1295

2 October 1978

Box 5964

Tucson, AZ 85703

(602) 623-0578

9, the slopes on top and bottom of the basalt seem to vary, at least locally, and the basalt roughly doubles in average thickness from about 54 feet in the wells toward the west to over 100 feet in these three wells, and 86 feet at Well 6 about one mile away toward the northwest.

Latest (most recent) (1977-1978) Pump Testing Results:

		(Relative)			Cala Viold
Year	Well #	Rank	Feet Draw Down	GPM	Gals. Yield Per Foot Draw Down
1978	1	2	133.67	1000	7.48
1978	2	1	132.	1000	7.58
(1977)	3	7	256.	735	2.87
1978	4	8	295.	800	2.71
1978	5	5	241.3	1100	4.56
1978	6	4	252.75	1175	4.65
1978	7	3	195.	1250	6.41
1978	8	9	318.	800	2.52
1978	9	6	281.	1170	4.16
	First, ((Original) or	Oldest Pump Testi	ing Records o	n Hand:
1968	1	3	106.3	1100	10.35
1968	2	2	102.67	1230	11.98
1974	3	9	233.5	520	2.23
1968	4	7	229.67	900	3.92
1968	5	6	204.	950	4.66
1964	6	4	221.	1400	6.33
1971	7	1	134.5	1718	12.77
1974	8	8	320.	1250	3.90
1976	9	5	314.	1500	4.78

10/5/78 Mpire 4-8416 Tought SAZ Matthewes, P.D. I Will Beveloped. Gravel packed. formation qui suje Sceen size.

Hais, Sird we 712-1613

